

Force and Settlement Characteristics of an Embankment on Soft Consolidating Soil with Lime Columns

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Abstract: Embankment supported on soft soil with lime column is analyzed. The lime column is modeled using a two noded plane frame element with three degrees of freedom (DOF) (two translational and one rotational), whereas the soil stress- strain behavior is presumed to be non-linear and modeled by a Cam-Clay model. The properties of lime column is obtained from a stress- strain curve obtained from a laboratory test. It is concluded from the study that addition of lime column in soft soil reduces the settlement after the construction.

Keywords: lime column, Embankment on soft consolidating soil, finite element analysis, ground improvement technique.

I. INTRODUCTION

Constructing embankments on soft soil with a high level of groundwater results in a rise in stresses leading to strain which causes settlement of stratum. Such settlement is due to a reduction in soil mass volume. When in a fully saturated soil environment, water in voids and soil particles are presumed to be incompressible, volume reduction takes place due to removal of water and this contributes to high lateral pressures and movement. Over settling, slopes and bearing failures, which resulting in delay in construction. In such situations, soil improvement techniques are often used to improve stability and reduce the time of consolidation on a soft soil with high ground water. Stability and consolidation time are two main factors in designing and building the foundation on soft soil. If a lime column is inserted vertically in the ground, it can significantly shorten the drainage path of soft clay reservoirs and significantly increase ground rigidity and strength in a short time. Lime columns are now regarded as the most cost-efficient ground improvement techniques worldwide, where soft compressible clay construction is unavoidable and have been applied in soft soil techniques such as embankment construction on soft soil. This system consists of a lime column which is designed to support embankment filling and reinforce the ground with designed length and diameter. Fig.1 shows the lime column inserted into the ground which supports the embankment fill for strengthen and stiffen soft soil having high ground water level. In recent years, there has been considerable development in understanding the behavior of embankment resting soft consolidating soil with a high groundwater level. Zheng et al. studied CFG-lime composite field pile construction theory and implementation [1]. The

two-dimensional geo-synthetic reinforced embankment on deep mixed column has been analysed by Han et al [2]. Huang Analyzed deep mixing columns under two and three dimensional embankments [3]. Geotextile reinforced embankment analyzed using computational and analytical tools over deep mixed columns by Madhyannapu [4]. 3-dimensional finite element analysis conducted on the CFG column of composite structures of different lengths and diameters by Zheng et al [5]. Simulations of Finite elements to model the multi-column support road embankment done by Abusharar et al [6]. Kumar et al made analytical study on Coir fiber drain's performance for enhancing soft ground in the construction of embankments [9]. Some of the experimental studies are carried out with randomly distributed coir fibers improved the strength of the soil by sarvade et al [8]. In the present work the effectiveness to use lime columns for the embankments on soft soil is studied

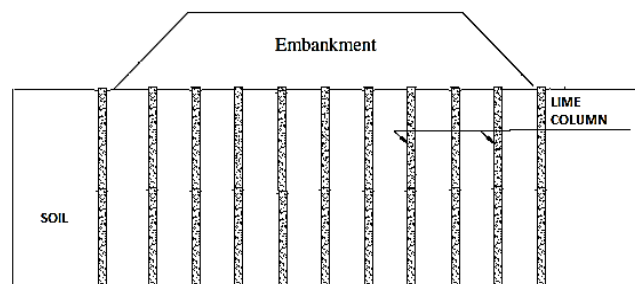


Fig. 1.Embankment supported by lime column

II. MODELING OF SOIL WITH LIME COLUMN

The model of an embankment supported on lime column considered for the analysis as shown in Fig.2. As shown in the figure, the embankment has crest width of 11 m, height of 2 m with side slope 1:1. It is supported on a lime columns of length of 5m and diameter 0.5m. The lime columns are modelled using a two noded plane frame element with three DOF (two translational and one rotational) whereas, the soil is modelled using a four noded plane strain quadrilateral element. Only the half of the embankment and the soil for study is taken into account because of symmetry.

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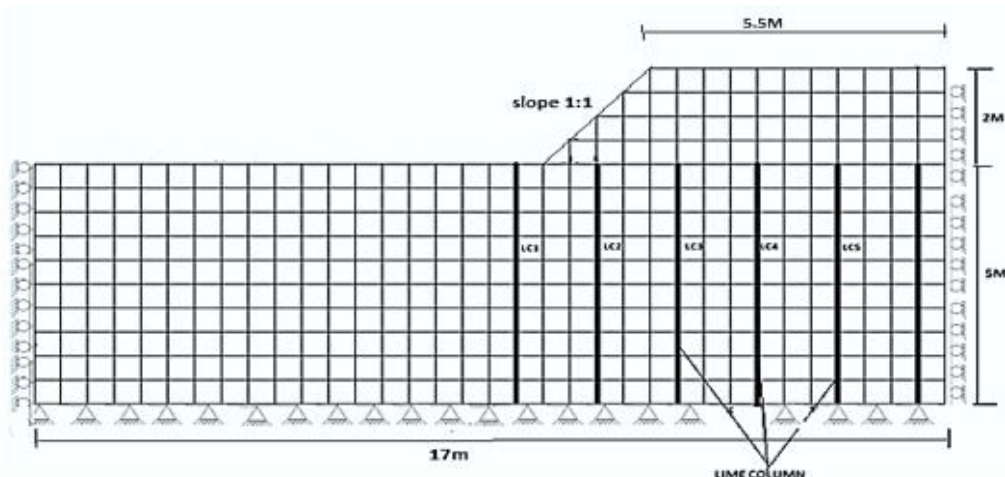


Fig. 2. Finite element model of an embankment supported on lime column

Horizontal displacement at the sides is restricted, while the horizontal and vertical displacement at the bottom of the soil are restricted. It is assumed that the embankment is constructed in stages. The soil stress- strain behavior is presumed to be non-linear and modeled by a Cam-Clay model. The properties of soft soil, embankment and lime column are shown in table 1 and 2, the settlement profile of the ground below the embankment and the axial forces on the lime column are obtained at various time intervals from end of construction till the end of consolidation.

Table 1 Material property of embankment and soft soil

MATERIAL PROPERTIES	
Elastic modulus, E , kN/m^2	5000
Poisson's ratio, μ	0.25
Unit weight, γ , kN/m^3	17
Effective cohesion, c' , kN/m^2	0
Angle of internal friction ϕ'	30°

Table 2 Material property of lime column

MATERIAL PROPERTIES	
Elastic modulus, E , kN/m^2	2×10^4
Poisson's ratio, μ	0.15

III. RESULTS AND DISCUSSION

The settlement profile of the ground below the embankment and the axial forces on the lime column are obtained at various time intervals for the embankment on soft soil with and without lime column and are compared in order to study the effectiveness of providing lime column as ground improvement technique. Fig.3 shows the settlement profile of the ground below the embankment at the end of consolidation. The variation of axial forces in lime columns 1 and 5 with depth at the end of construction and at the end of consolidation are shown in Figs. 4 and 5. From these figures, it can be observed that

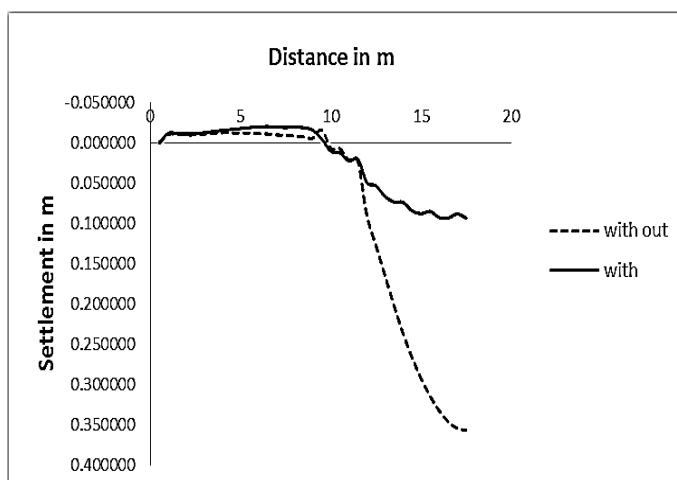


Fig. 3. Settlement profile of the ground below the embankment with and without lime column at the end of consolidation

Axial forces on the lime column 1 and lime column 5 at the end of construction and at the end of consolidation indicates that the forces in both the lime columns are lesser immediately after the construction of embankment compared to the forces in the lime columns at the end of consolidation. This is because, immediately after the construction, the major load of the embankment is transferred to the pore fluid and the load transferred to the soil and columns are less. However, as the time increases, the pressure in pore fluid decreases due to consolidation and the load transfers to the soil mass and lime columns. Thus there is a significant increase in the axial force in the lime columns at the end of consolidation compared to the load at the end of construction. Also, it can be observed that the load on column 1 increases with depth reaches a maximum value at about 3 m from the ground surface and then decreases as the depth increases both at the end of construction and end of consolidation.

However, for the lime column 5 provided below the center of the embankment the axial load is minimum near the ground surface and increases with the depth.

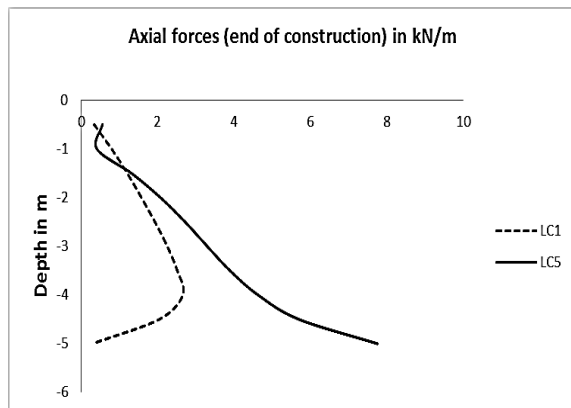


Fig. 4. Axial forces on the lime column at the end of construction.

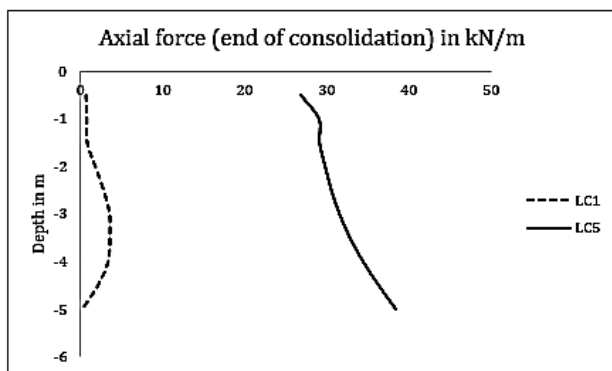


Fig. 5. Axial forces on the lime column at the end of consolidation.

IV. SUMMARY AND CONCLUSIONS

Lime column performance is analyzed using the finite element approach in order to improve soft soil below the embankment. The study draws the following conclusions

- Addition of lime column in soft consolidating soil reduces the settlement after the construction of embankment.
- The axial force in the lime column is lesser immediately after the construction of embankment because major load of the embankment is transferred to the pore fluid.
- The axial force in the lime columns increases significantly at the end of consolidation compared to the load at the end of construction.

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