BattLeDIM 2020

Battle of the Leakage Detection and Isolation Methods

Problem Design and Results Evaluation

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BATTLEDIM Organizing Committee

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The Challenge



The L-Town water distribution network

- Created based on a real network of a city in Cyprus
- ~10000 people
- 42 km pipe length
- Map dimensions:2.6 km height, 3 km width
- 782 Junctions, 2 Reservoirs, 1 Tank
- 905 Pipes, 1 Pump, 3 PRVs
- Pipe segments of ~50 meters



Base demands

- Created based on the real network:
 - ➤ Each node is assigned a polygon
 - Find Building Area (BA) in polygon from real network (open data)
 - > Type of consumer in each building:
 - > Residential
 - Commercial
 - > Industrial
 - \triangleright Base demand of node i:

$$d_i^b = \sum_j (T_j * CBA_j) * BA_i$$

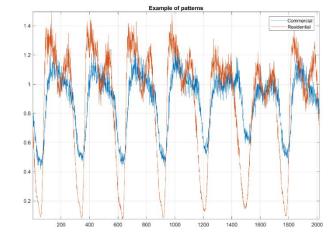
 $*T_i$: Percentage of each consumer type in polygon

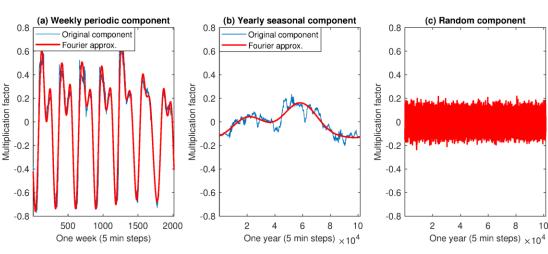
*CBA: Consumption per building area for consumer type j



Demand Patterns

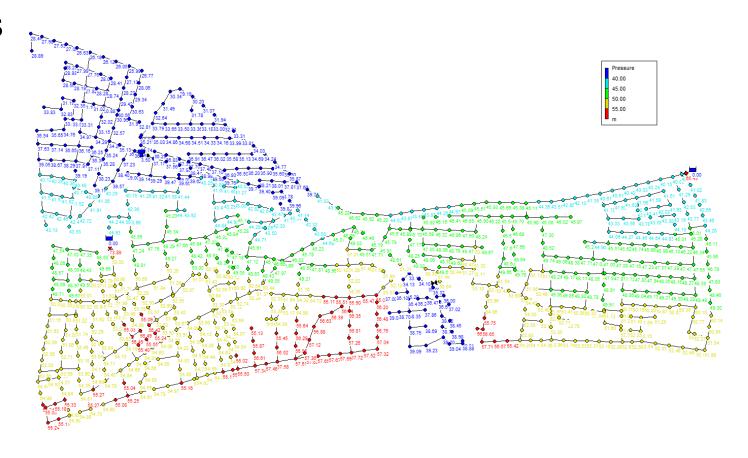
- Two nominal patterns: Residential and Commercial
- Extracted from real data
- Each node is assigned a **unique demand pattern** randomly generated as described in [1]
- Industrial patterns based on real AMR data
- Duration: Two years, 5 minute steps
- Seasonal and weekly periodicity
- Peaking factor (MDD/ADD) matching the size of the system
- No special days





Network behavior

- Different pressure areas
- Normal pressure during fault-free operation is between 2.5-6 bar.
- Tank is typically refilling during night and releasing water during the day.
- Pressure-driven demands solved using WNTR [2]



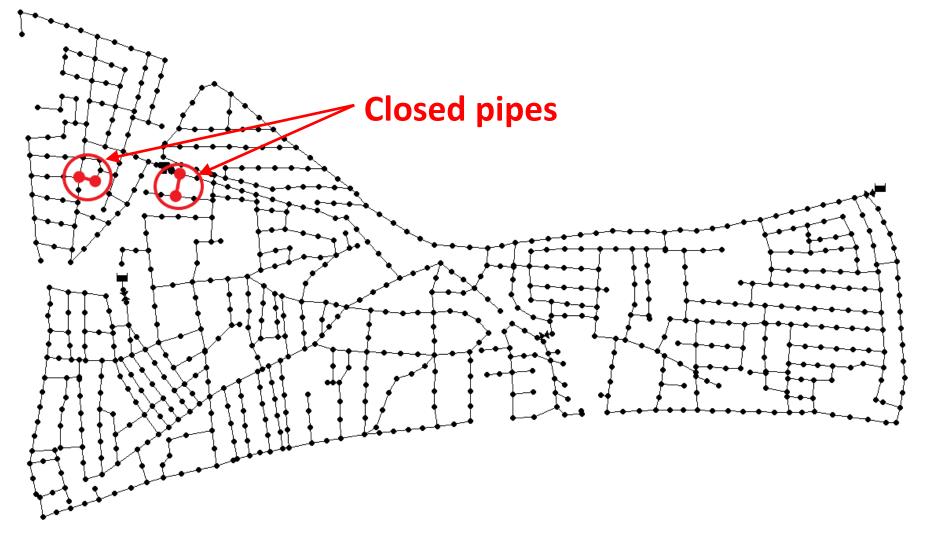


Nominal model and Real network

- "Real" network is used for generating the data
- Nominal model contains the "available" network information (provided as an EPANET input file (.inp))
- Differences of nominal model compared to real network:
 - Base demands: randomized uniformly between $\pm 10\%$ of real value
 - **Demand patterns:** Nominal residential and commercial patterns available. Industrial patterns not available.
 - Pipe parameters: randomized uniformly $\pm 10\%$ of real value (Elevation and pump curve uncertainty were omitted to reduce problem complexity)
 - Topology: "p37" and "p251" closed in real network

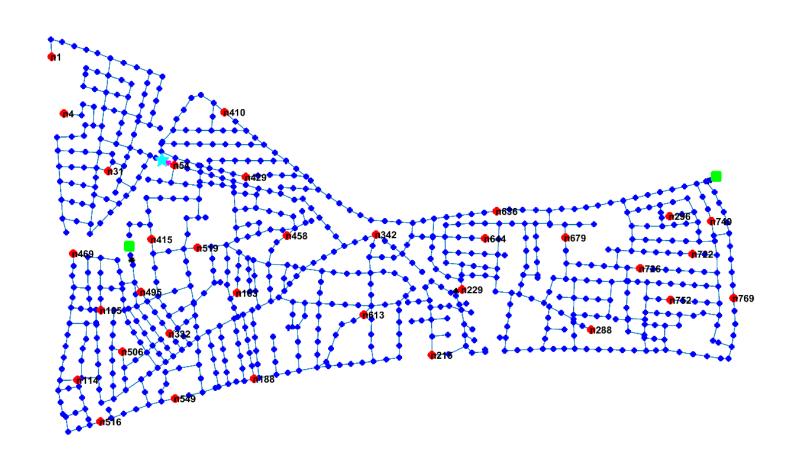


Nominal model and Real model



Sensors

- One (1) tank water level sensor
- Three (3) flow sensors (Pump and DMA entrances)
- Thirty-three (33) pressure sensors
 - Placed using a sensitivity matrix method
- Automatic Meter Reading devices (AMRs) in "Area C" (red)
- Industrial consumers (green)



The datasets

- Sensors give accurate readings with no time-delay (reduce problem complexity)
- Readings rounded to 2 decimal points
- ➤ Historical dataset:
 - One year (2018) of sensor data
 - The **time and repair location** of pipe bursts that have been fixed are provided
- > Evaluation dataset:
 - One year (2019) of sensor data

All	Α	В	С	D	E	
1	Timestamp	n1	n4	n31	n54	n
2	1/1/2019 0:00	28,63	33,72	37	36,93	
3	1/1/2019 0:05	28,66	33,75	37,02	37,05	
4	1/1/2019 0:10	28,67	33,76	37,04	37,03	
5	1/1/2019 0:15	28,69	33,78	37,05	37,09	
6	1/1/2019 0:20	28,68	33,77	37,05	36,95	
7	1/1/2019 0:25	28,72	33,8	37,07	37,08	
8	1/1/2019 0:30	28,74	33,82	37,09	37,18	
9	1/1/2019 0:35	28,73	33,82	37,09	37,09	
10	1/1/2019 0:40	28,76	33,84	37,12	37,15	
11	1/1/2019 0:45	28,76	33,85	37,13	37,14	
12	1/1/2019 0:50	28,8	33,88	37,15	37,22	
13	1/1/2019 0:55	28,82	33,9	37,17	37,27	L
14	1/1/2019 1:00	28,83	33,91	37,18	37,22	L
15	1/1/2019 1:05	28,85	33,93	37,2	37,27	L
16	1/1/2019 1:10	28,86	33,94	37,21	37,26	
17	1/1/2019 1:15	28,89	33,96	37,23	37,32	
18	1/1/2019 1:20	28,89	33,97	37,24	37,32	
19	1/1/2019 1:25	28,91	33,98	37,25	37,39	
20	1/1/2019 1:30	28,93	34	37,27	37,38	
21	1/1/2019 1:35	28,93	34,01	37,28	37,36	
22	1/1/2019 1:40	28,95	34,02	37,29	37,41	
23	1/1/2019 1:45	28 97	34 04	37 3	37 41	



The Leakages



Leakages are assumed **the only faults** that exist in L-Town.

(Pipe bursts as well as background leakages)



Leakage modeling

• Leakages are modeled on pipes as described by Crowl and Louvar [3]:

$$d_{leak}(t) = C_d A(t) p^a(t) \sqrt{2/\rho}$$

- In BattLeDIM:
 - Leak demand $d_{leak}(t)$ $(\frac{m^3}{h})$
 - Leak hole area A(t) (m^2)
 - Pressure p(t)(m)
 - Discharge coefficient $C_d = 0.75$ (assuming turbulent flow)
 - Discharge exponent $\alpha=0.5$ for steel pipes and $\alpha=2.5$ for plastic pipes



Leakage types

Three types of leaks categorized depending on their magnitude:

- 1. Background leaks: Small leaks with size of 1-5% of the average inflow.
- 2. Medium pipe bursts: Pipe breaks with size of 5-10% of the average inflow.
- 3. Large pipe bursts: Pipe breaks with size above 10% of the average inflow.
- \triangle Average system inflow $\sim 180m^3/h$

Large and some medium leakages (above $15 \, m^3/h$) are **fixed by the water utility** after a reasonable amount of time selected in random (max 2 months)



Leakage time profile

1. Abrupt leakages (pipe bursts):

$$A(t) = \begin{cases} 0 & t < t_{start}, t > t_{end} \\ A_{max} & t_{start} \le t \le t_{end} \end{cases}$$

2. Incipient leakages (background leakages which can evolve into bursts):

$$A(t) = \begin{cases} 0 & t < t_{start}, t > t_{end} \\ A_{max} \left(\frac{t - t_{start}}{t_{peak} - t_{start}} \right) & t_{start} \le t < t_{peak} \\ A_{max} & t_{peak} \le t \le t_{end} \end{cases}$$

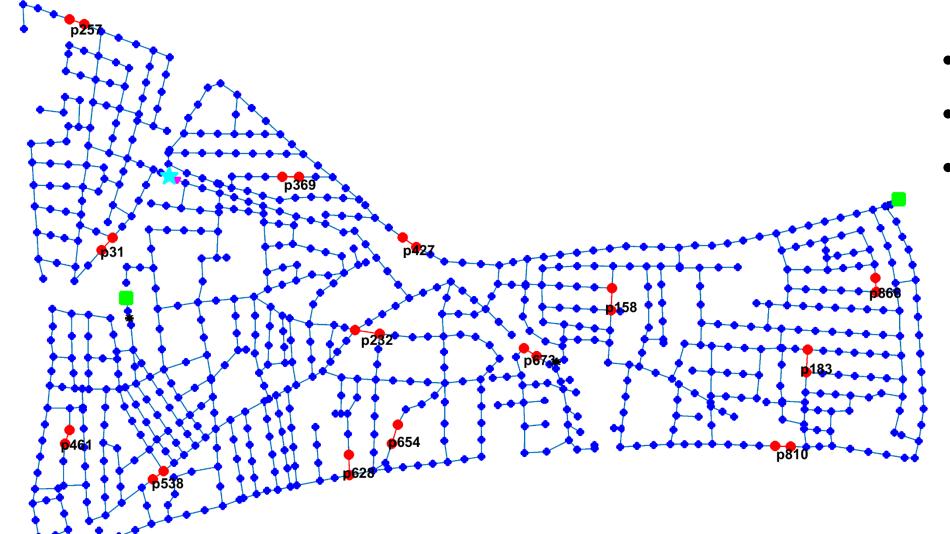


Leakage placement

- An algorithm places the leakages randomly, with constraints:
 - Maximum number of leakages in a year (20)
 - Minimum time (2 weeks) between leakages with overlapping detection radius to ensure separability
 - Detection radius (300 meters)
- Leakage characteristics are selected randomly, with constraints:
 - Predetermined number of background, medium and large leakages
 - Predetermined number of abrupt and incipient leakages
 - Incipient peak time selected based on the leakage type and magnitude

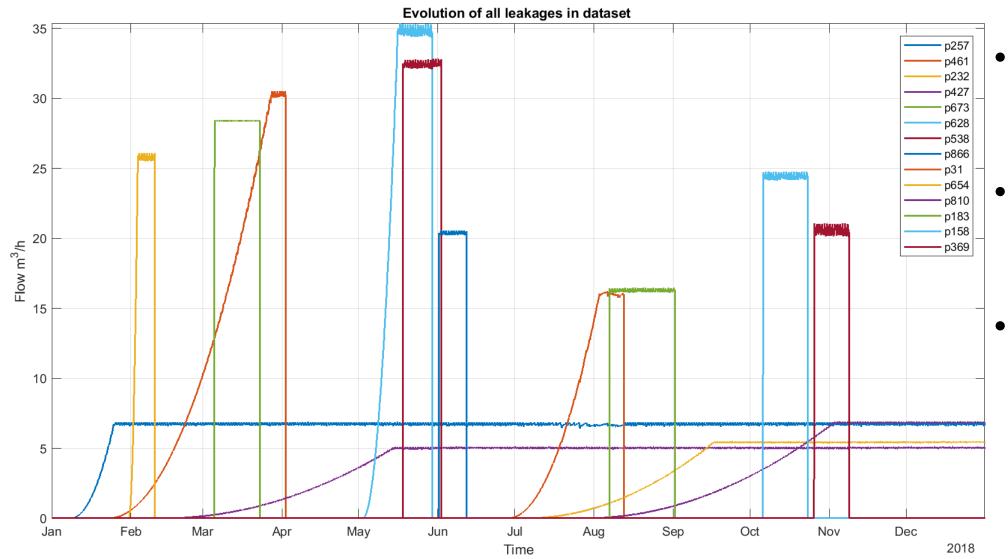
Leakage Locations - Historical Dataset 2018

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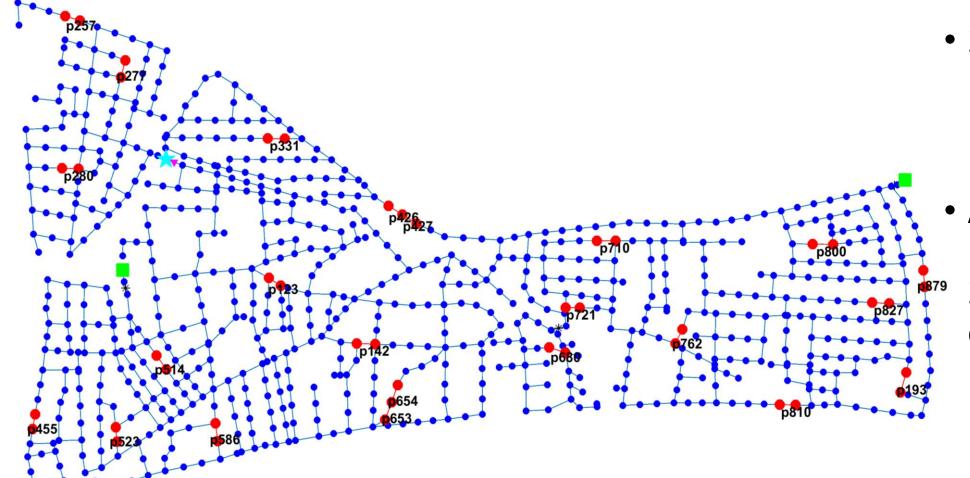
- 14 Leakages total
- 10 are fixed (given)
- 4 background leakages are not fixed and continue into 2019 on pipes p257, p427, p810, p654

Leakage Evolution - Historical Dataset 2018



- Abrupt and incipient leakages
- Mainly large leakages (above $15 m^3/h$
- Background leakages smaller than $8 m^3/h$

Leakage Locations - Evaluation Dataset 2019

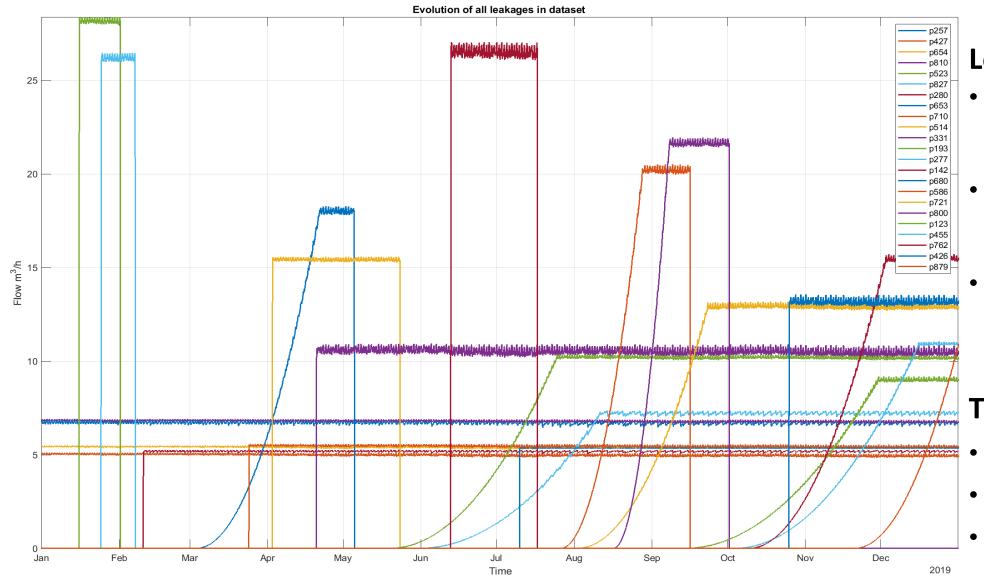


- 23 Leakages total:
 - 4 begin in 2018
 - 19 begin in 2019
- All the leakages present in the
 2019 dataset are evaluated

Leakage Evolution - Evaluation Dataset 2019

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Leakage Size:

Background leaks

Count: 3+4

Max diameter: 1.2 cm

Medium pipe bursts

Count: 10

Max diameter: 1.8 cm

Large pipe bursts

Count: 6

Max diameter: 2.4 cm

Time profile:

- 10 incipient leaks
- 9 abrupt leaks
- 4 continuing from 2018



Evaluation methodology



Evaluation approach

Evaluation of participant results follows a **pure economic approach**:



- The water utility of L-Town calculates the profit from water saved from successful detections.
- 2. The utility also considers the **cost of the repair crew** every time it is sent to search for a leakage.



❖ Note: The selected methodology has been chosen by the organizing committee as the most suitable for this competition. Alternative evaluation methodologies have also been considered and will be described in a future journal paper.



Evaluation procedure (1/4)

Set of rules are applied to each result to **calculate the score**:

- 1. True detection (**True Positive**) condition:
 - A detection pointing at a time during the lifetime of a leakage and within a predefined pipe length distance from the leak location
- 2. False detection (False Positive) condition:
 - Detections which do not satisfy the *True detection condition* (1.) above.
- 3. Missed detection (False Negative) condition:
 - Leakages in 2019 dataset which have not been detected



Evaluation procedure (2/4)

- 4. Detections are evaluated in **chronological order**, i.e., from the earliest detection to the latest detection given.
- 5. Detections outside 2019 are ignored (not penalized).
- **6.** Repeated detections of the same leakage are ignored.
- 7. A single detection may detect only one leakage, even if more than one leakage is in the detection area. In the case of multiple detections, only the leakage closest to the detected link is considered to be detected successfully.



Evaluation procedure (3/4)

8. The **profit from water saved** p_w^i (euro) by a detection i, for a detected leakage j, given leakage flow rate $q^j(t)$, and cost of water per cubic meter c_w (euro), is calculated as follows:

$$p_w^i = \left(\sum_{k=t_d^i}^{t_{end}^j} q^j(k) * \Delta t\right) * c_w,$$



9. The **utility repair crew cost** c_r^i for a given detection i, detection distance from leakage x_{ij} , maximum detection distance x_{max} , and maximum repair crew cost c_r is calculated as follows:

$$c_r^i = \begin{cases} -\left(\frac{x_{ij}}{x_{max}}\right) c_r & x_{ij} < x_{max}, \\ -c_r & x_{ij} \ge x_{max} \end{cases}$$



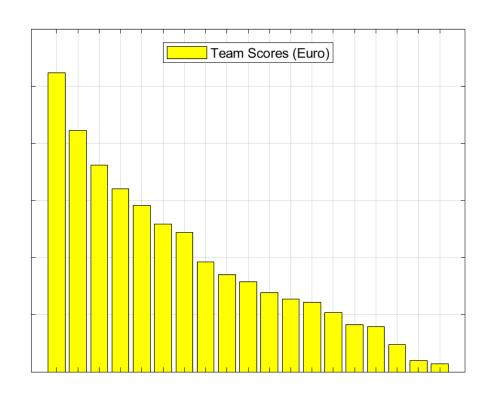
Evaluation procedure (4/4)

10. The **score per detection** for a given detection i is given by adding the profit and cost:

$$s^i = p_w^i + c_r^i$$

11. The **total score** *S* for a given set of detections is given by:

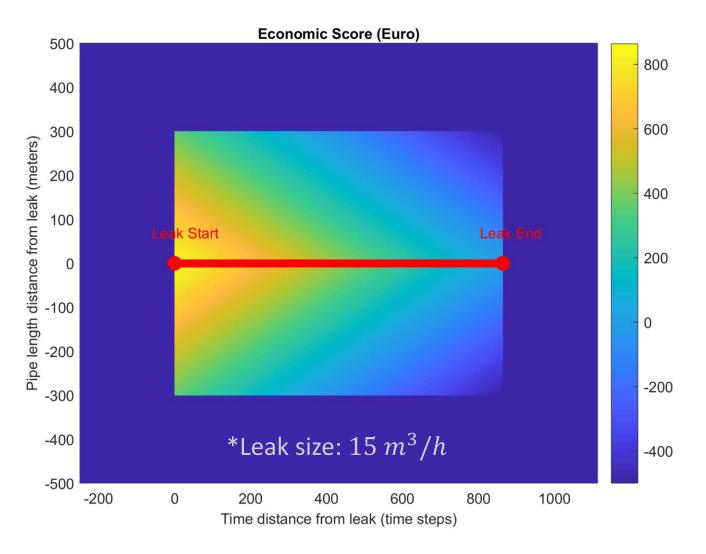
$$S = \sum_{i} s^{i}$$



Scoring function

BattLeDIM evaluation parameters:

Max detection distance (x_{max}) :	300 (meters)		
Cost of water per m^3 (c_w):	0.80 (Euro)		
Maximum repair crew cost (c_r) :	500 (Euro)		



Evaluation code

Openly available on GitHub: https://github.com/KIOS-Research/BattLeDIM

*Instructions are provided on how to score your results file

Run online using CodeOcean: https://codeocean.com/capsule/8332511

Pending approval

More code, available shortly:

- 1. Generation of BattLeDIM network code
- 2. Sensor placement code
- 3. Leakage placement code

