# EFFECT OF GEOPOLYMER MORTAR IN FERROCEMENT FOR VARIATION IN MESH SIZE AND NUMBER OF LAYERS

KHALIL S. SHAIKH

PG Scholar, Department of Civil Engineering Dr.Vithalrao Vilke Patil College of Engineering Ahmednagar, Maharashtra, India Email-shaikhkhalils@gmail.com

# PROF. P.B. AUTADE

PG Guide, Department of Civil Engineering Dr.Vithalrao Vilke Patil College of Engineering Ahmednagar, Charashtra, India Email- pankajautade@gmail.com

#### **ABSTRACT:**

worlds Portland Globally, the cement production contributes about 1.6 billion tons of CO2 or about 7% of global loading of CO<sub>2</sub> into the atmosphere. The manufacture of Portland cement releases Carbon dioxide that is significant contributor of Green house gas emissions to the atmosphere. Geopolymer is me eco friendly material and a strong alternat Portland cement. Geopolymer mortar can b ised in Ferro cement instead of other conventional cials like cement mortar. Ferro cement is a com material formed by closely spaced wire mesh wh uses wire meshes as reinforceme rocement ha high tensile strength, minim s, ease of in the mould ability. Experiment investigat has been carried out to study the Geopoly r mortar in Ferrocement for variation in h size, sile testing layers. For this, done on ferrocement specific of size 75 60 x 30 mm ouble, trip reinforced with sing ver *Jusing* variation in meshes of ferent size hilarly, ecimens compre testing of cubi size 70 x double, triple layer 70 x 70 m inforced with sin ferent sizes was of meshes o lone. Test results show that te and co sive strength of ith increase in number of layer specimen's increa of meshes and mesh lse played an important role in strengthening of spe nens. Compressive strength of single mesh layer in Geopolymer mortar is greater than single mesh layer in conventional cement mortar by approximately 5 %. For double layer mesh, specimens with Geopolymer mortar show greater strength than specimens with conventional cement mortar by 5-6 %.

KEYWORDS: Ferrocemnt, Flyash, Geopolymer, Tensile strength, Compressive strength.

## INTRODUCTION:

of production of carbon of The rat e released to the phere during the production of P atm ad cement and ash. a bv-p duct from thermal power stations easing with the increasing demand on ide is ij and hence needs proper infra development action to minimize the impact on the attentio tainabili our living vironment. De-carbonation of ln d ring manufacturing of cement is estone in t eration of one ton of carbon dioxide oonsible for th the atmosphere for each ton of Portland cement, as can be seen from the following reaction equation :

 $CaCO_3 + 2SiO_2 3CaO.SiO_2 + 2 CaO.SiO_2 + 5 CO_2$ . The current ribution of green house gas emission from the d cement production is about 1.35 billion tons ually or about 7% of the total greenhouse gas missions to the earth's atmosphere[1]. Furthermore, Portland cement is also among the most energy-intensive construction materials, after aluminum and steel. Geopolymer concrete is a material that does not need the presence of Portland cement as a binder. Instead, the source of materials such as fly ash, that are rich in Silicon (Si) and Aluminium (Al), are activated by alkaline liquids to produce the binder. Hence, concrete with no cement. Geopolymer is produced without the presence of Portland cement as a binder; instead, the base material such as fly ash, that is rich in Silicon (Si) and Aluminium (Al), is activated by alkaline solution to produce the binder. The Geopolymer concrete possesses high strength, undergoes very little drying shrinkage and moderately low creep, and shows excellent resistance to sulphate attack[3][4][5].

Ferrocement is a material of construction having great variety, which possesses unique structural properties. It is a composite formed with closely wire mesh tightly wound round skeletal steel and filled with rich cement mortar. Welded mesh, mild steel angles or bars are used for forming skeleton, while chick enmesh, square mesh or expanded metal are used as mesh reinforcement. Mortar

mix may be (1:1.5) to (1:4) by volume[2]. It combines the properties of thin sections and high strength of steel, mouldability of concrete, lightweight and eases of working of timber, high tensile strength capacity of prestressed concrete and crack control of fiber reinforced concrete. Ferrocement can replace all these materials. In addition it needs no formwork or shuttering for casting. Ferrocement has applications in all fields of civil construction, including water and soil retaining structures, building components, space structures of large size, bridges, domes, dams, boats, conduits, bunkers, silos, treatment plants for water and sewage and chimneys partially.

#### **OBJECTIVES OF INVESTIGATION:**

> To study Tensile characteristics of Geopolymer based Ferrocement reinforced with wire meshes of different sizes in different layers .

> To compare the behaviour under Compression of cement mortar based Ferrocement and Geopolymer mortar based Ferrocement.

#### **MATERIALS:**

The present research work is experiment requires following materials.

1. Cement: The cement used in this experime rk is "ACC 43 grade Ordinary Portland Cement". All proof cement are tested by referring IS 8112 -Specification for 43 Grade Ordinary Portland Cement. 2. Fine aggregate: Locally available conforming TIVE to Grading zone II of IS: 383-2 /0. Finene odulus was found to be 2.76, Specific gr as 2.59. 3. Fly ash-Fly Ash is available pow n and is procured from Dirk ia Pvt. Ltd., It is ave 30Kg bags, color er the product is light gra Part 1-2003 name "Pozzocrete 63" rming to IS: as mineral admixture in dry der form.

4. Water by table water availation laboratory is used.

5. Sochum en soxide: Sodium hydroxide available in flakes form is used. In this investigation the odium hydroxide of 13M concentrate under used.

6. Sodium Silicate  $(1000)_3$  Sodium Silicate also known as water glass or liquid glass wailable in liquid (gel) form. In present investigation sodium silicate in gel form is used.

7. Wire meshes: We meshes generally used in ferrocement structures are having opening sizes in mm as 25 X 25,50 X 50, 75 x 75, 100 x 100, and 150 x 150.The wire gauges may vary from 10 to 18.

8. Tension Test Mould: The mould has been prepared by using ISA  $30 \times 30 \times 3$  mm, Two angles of 750mm length are placed on metal sheet with screw arrangement so as the spacing between faces of these angles remains equal to the width of specimen i.e. 60mm.The size of mould used in this

project is  $750 \ge 60 \ge 30$  mm. Total 6 numbers of Moulds are prepared for casting.



Fig. 1 - Tension Test Mond

9 compression Test Mould: Cubical moulds of size 70 x 70 x 00 m were vice to conduct compression test. Total 6 numerical of moulds were used for casting of specimens. All the mould before properly of the before filling with cement fortar.

## THODOLOGY

The fresh dy ash-based geopolymer mortar was dark in colour (due to the dark colour of the fly ash), and was cohesive. The amount of water in the mixture played important role on the behavior of fresh mortar. Notice (2002) suggested that it is preferable to mix the so rum silicate solution and the sodium hydroxide solution together at least one day before adding the liquid to the solid constituents.

1. Mix sodium hydroxide with water at least one day prior to adding the liquid to the dry materials.

2. Mix all dry materials for about three minutes by hand mixing. Add the liquid component of the mixture at the end of dry mixing, and continue the wet mixing for another four minutes.

Ratio of sodium silicate solution-to-sodium hydroxide solution, by mass, can be used in the range of 0.4 to 2.5. But this ratio was fixed at 1 for most of the mixtures because the sodium silicate solution is considerably cheaper than the sodium hydroxide solution

Preparation of Binder Solution

Binder solution plays a vital role in the binding of the fly ash based geopolymer mortar. Binder solution is a mixture of Sodium Hydroxide and Sodium Silicate. In this investigation

the sodium hydroxide flakes in 13 molar concentrations were used. Binder solution is mixed 24 hours prior to the mixing of mortar.

## **TESTING PROGRAM:**

**Tensile Strength Test** (IS 516-1959): All the specimens were tested on Universal Testing Machine. In order to test the specimen, tensile test setup was prepared. Gauge lengths were marked on the each specimen and for proper arrangement rubber grip were used. Load was applied gradually through a hydraulic system and displacements were recorded.



Fig. 3 - Tension Test Setup

Table 1 - Single Mesh Tensile Strength						
Sample	Notation	<b>Opening Size</b>	Mortar	Insile		
		of Mesh	Material	Strength		
		(in x in)	•	(N/mm²)		
1	S X	-	СМ	0.264		
2	S 1	0.50 x 0.50	СМ	2.23		
3	S 2	0.75 x 0.75		2.04		
4	S 3	1.0 x 1.0	GM	2.96		
5	S X	-	GPN	0.271		
6	S 1	0.56 x 0.50	GPM	2.30		
7	S 2	0.75 x 0.75	GPN	2.15		
8	S 3	1.0 x 1.0	GF	2.98		

Table 1 shown result for shown mesh tensile strength of various spectrums. It is objected tensile strength of specimen 3 too maximum mength as compared together specimens.



Table 2 – Double Layer Mesh Tensile Strength					
Sample	Notation	<b>Opening Size</b>	Tensile		
		of Mesh	Material	Strength	
		(in x in)		(N/mm²)	
1	S 1	0.50 x 0.50	СМ	5.70	
2	S 2	0.75 x 0.75	СМ	3.98	
3	S 3	1.0 x 1.0	СМ	5.81	
4	S 1	0.50 x 0.50	GPM	5.75	
5	S 2	0.75 x 0.75	GPM	3.88	
6	S 3	1.0 x 1.0	GPM	5.92	

Table 2 shows result to houble layer mesh tensile strength of various spramens. It is observed tensile strength of speciment meases with number of layer of meshes.



Graph 2 - Double Layer Mesh Tensile Strength

## Table 3 – Triple Layer Mesh Tensile Strength

Sample	Notation	Opening Size	Mortar	Tensile
		of Mesh	Material	Strength
		(in x in)		(N/mm <sup>2</sup> )
	S 1	0.50 x 0.50	СМ	8.02
2	S 2	0.75 x 0.75	СМ	5.08
3	S 3	1.0 x 1.0	СМ	7.71
4	S 1	0.50 x 0.50	GPM	8.26
5	S 2	0.75 x 0.75	GPM	5.36
6	S 3	1.0 x 1.0	GPM	7.62

Table 3 shows result for triple layer mesh tensile strength of various specimens. It is observed triple layer mesh tensile strength of specimens is more as compared to single layer & double layer specimens.





Table 4 – Combined Mesh Tensile Strength					
Sample	Notation	<b>Opening Size</b>	Mortar	Tensile	
		of Mesh	Material	Strength	
		(in x in)		(N/mm²)	
1	S 1	1.0 x 1.0 + 0.75	СМ	5.102	
		x 0.75			
2	S 2	0.75 x 0.75	СМ	4.972	
		+0.50 x 0.50			
3	S 3	1.0 x 1.0 + 0.50	СМ	5.463	
		x 0.50			
4	S 1	1.0 x 1.0 + 0.75	GPM	5.211	
		x 0.75			
5	S 2	0.75 x 0.75	GPM	5.011	
		+0.50 x 0.50			
6	S 3	1.0 x 1.0 + 0.50	GPM	5.521	
		x 0.50			

Table 4 shows result for combined mesh tensile strength of various specimens. It is observed of all the various combinations, specimen having combination  $1.0 \times 1.0 + 0.50 \times 0.50$  gave more strength.



Graph 4 - Combined Menn Tensile Strongth

Compressive 1959): Strength IS 51 For Te compressive streng test, dimensions 70 x 70 mm wei ted . All cured specimens have been ted in con ion testing machine lines ment per the (IS The top surface specimen 516:1959 s leveled and After 24 hours specimens were demoulded an e transferred to c g tank wherein they r the age 7<sup>th</sup>,14<sup>th</sup> & were allowed t for 28 day Ał 28<sup>th</sup> days curing, cubes tested on Universal testing machine Failure load / cross Compressive strength sectional area.



Fig.4 - Compression Testing

Table 5 - Single Mesh Compressive Strength					
Sample	Notation	Opening	Mortar	Tensile	
		Size of	Material	Strength	
		Mesh		(N/mm²)	
		(in x in)			
1	S X	-	СМ	42.2	
2	S 1	0.50 x 0.50	СМ	46.23	
3	S 2	0.75 x 0.75	СМ	44.8	
4	S 3	1.0 x 1.0	СМ	43.5	
5	S X	-	GPM	42.6	
6	S 1	0.50 x 0.50	GPM	47.04	
7	S 2	0.70.0.0	GPM	45.21	
8	S 3	1.0 x 1.0	GPM	44.02	

Table 5 shows require for compressive strength of specimens with angle mesh the observed that, specimen 1 has more compressive strength and ompared to others.



Graph 5 - Single Mesh Compressive Strength

Je 6 –	Double	Layer	Mesh	Compre	ssive St	rength

ample	Notation	Opening Size of Mesh (in x in)	Mortar Material	Tensile Strength (N/mm <sup>2</sup> )
1	S 1	0.50 x 0.50	СМ	53.45
2	S 2	0.75 x 0.75	СМ	50.78
3	S 3	1.0 x 1.0	СМ	48.75
4	S 1	0.50 x 0.50	GPM	55.21
5	S 2	0.75 x 0.75	GPM	52.14
6	S 3	1.0 x 1.0	GPM	49.86

Table 6 shows result for compressive strength of specimens with double layer mesh. It is observed that, compressive strength of specimens increases with increase in number of layer of meshes.



Graph 6 - Double Layer Mesh Compressive Strength

Table 7 – Triple Layer Mesh Compressive Strength					
Sample	Notation	<b>Opening Size</b>	Tensile		
		of Mesh	Material	Strength	
		(in x in)		(N/mm²)	
1	S 1	0.50 x 0.50	СМ	62.3	
2	S 2	0.75 x 0.75	СМ	58.1	
3	S 3	1.0 x 1.0	СМ	54.4	
4	S 1	0.50 x 0.50	GPM	65.1	
5	S 2	0.75 x 0.75	GPM	60.3	
6	S 3	1.0 x 1.0	GPM	56.6	

Table 7 shows triple layer mesh compressive strength of specimens is more as compared to single layer & double layer specimens.



Graph 7 - Triple Layer Mesh Compressive Stre

Table 8 – Combined Mesh Compressive strength					
Notation	Opening Size of Mesh (in x in)	Mortar Material	Tehene Strengen (N/mm <sup>2</sup> )		
\$ 1	1.0 x 1.0 0.75 x 0.7		49.76		
S 2	0.75 0.75 +0.50 0.50	СМ	52.11		
S 3	1.0 x 1. 0.50 x 0.50	CN	<u>51.1</u> 0		
S 1	1.0 x 1.0 +	ORM	51.35		
S 2	+0.50×0.75	GRW	53.56		
\$ 3	1.0 x 1.0 0.50 x 0.50	GPM	52.87		
	Notation   S1   S2   S3   S1   S2   S3   S1   S2   S3	NotationOpening Size of Mesh (in x in)S1 $1.0 \times 1.0$ $0.75 \times 0.7$ S2 $0.75 \times 0.7$ S2 $0.75 \times 0.75$ $+0.5$ S3 $1.0 \times 1.0$ $0.50 \times 0.50$ S1 $1.0 \times 1.0$ + $-75 \times 0.75$ S2 $0.75 \times 0.75$ $+0.5 \times 0.75$ S2 $0.75 \times 0.75$ $+0.5 \times 0.75$ S3 $1.0 \times 1.0$ + $-75 \times 0.75$ S2 $0.75 \times 0.75$ $+0.5 \times 0.75$ S3 $1.0 \times 0.50 \times 0.50$	NotationOpening Size of Mesh (in x in)Mortar MaterialS1 $1.0 \times 1.0$ $0.75 \times 0.70$ $1.0 \times 1.0$ $0.75 \times 0.75$ S2 $0.75 \times 0.75$ $+0.75 \times 0.75$ CM CM CMS3 $4.0 \times 1.0$ $0.50 \times 0.50$ CM CM CM CMS1 $1.0 \times 1.0 +$ $1.0 \times 1.0 +$ $75 \times 0.75$ CM CM CM CMS2 $0.0 \times 0.50$ CM <b< td=""></b<>		

Table & the vertices combinations, specimen having termination  $40.05 \times 0.75 + 0.50 \times 0.50$  gave more strength.



Graph 8 - Combined Mesh Compressive Strength

#### ACKNOWLEDGEMENT:

Experimental work was carried out using the facilities in Civil Engineering Department laboratory of P.D.V.V.P.COE, Ahmednagar. I sincerely wish to thanks my guide & ME Co-ordinator Prof. P.B. Autade and HOD Prof. U. R. Kawade, for their valuable Suggestions and authorities for their kind support. I also wish to thank the laboratory staff for their help and support during experimental work.

## CONCLUSIONS:

▶ It is concluded that for samples with two layers of mesh the increase in the strength is observed to be in the range 96% - 140% as compared to single layer. Further using 3 layers hareases tension trength in the range of 172% - 260% as compared to single haver.

is concluded that increa.  $\triangleright$ tensile strength of s having mesh size 0.5" x 0.5" w avers is 33% sam more as co pared to samples beying mesh sizes 50 0.75" an 1" with 3 layers. Similarly, increase th of sample having mesh size 0.5" x 0.5" in te is 66% - 67% pure as compared to samples es 0.75" x 0.05" and 1" x 1" with 2 layers. with 2 ving mes

Generative Ferrotement structures are exposed to usion effect being adverse condition they may be abjected to compression effect too. From above studies it was found that Ferrocement can also be very much effective under compression as result obtained showed rease in compressive strength with increase in number others of meshes.

It is concluded that for various combination of meshes used, combination of mesh  $0.75 \times 0.75 + 0.50 \times 0.50$  gives more compressive strength as compared to other two combinations.

▶ It is also concluded that Compressive strength of single mesh layer in Geopolymer mortar is greater than single mesh layer in conventional cement mortar by approximately 5%. For double layer mesh, specimens with Geopolymer mortar shows greater strength than specimens with conventional cement mortar by 5-6 %.

## **REFERENCES:**

- 1) (BESS SB 13-Page-159) Building Enclosure Sustainability Symposium, California
- 2) Divekar B.N., 'Ferrocrete Technology- Developments in Pune region'.
- 3) Davidovits J, High Alkali Cement for 21<sup>st</sup> century concretes in concrete technology, Past, Present and Future.ACI SP-144.p.383-397
- Wallah, Hardjito and Rangan B.V.- Sulhate resistance of fly ash based Geopolymer concrete in the third millennium, The 21<sup>st</sup> Biennial conference of concrete of Australia.

- 5) Cheng T.W. and Chiu J.P, *Fire resistant Geopolymer produced by granulated Blast Furnace Slag*, Minerals engineering 2003,16(3),p.205-210.
- 6) Patankar and Jamkar, *Effect of Concentration of Sodium Hydroxide and Degree of Heat Curing on Fly Ash- based Geopolymer Mortar*, Indian Journal of Material Science, Volume 14, Article ID 938789.
- S. Nagan & R. Mohana "Behaviour of Geopolymer Ferrocment Slabs Subjected To Impact", ISJT, Volume 38, no C1, pp 223-233.
- 8) Chindaprasirt et al *"High Strength Geopolymer using Fine High Calcium Fly Ash"*, Journal of Materials in Civil Engineering, Volume 23, Issue 3, March 2011.
- 9) Mitali patil, Dr J.R. Patil et a "Effect of reinforcement orientation on compressive strength of Ferrocement and bitumen Ferrocement", IRJET, Vol 2, Issue 3, June 2015.

- 10) Swayambhu Bhalsing, Sayyed Shoaib, Pankaj Autade "Tensile Strength of Ferrocement with respect to Specific Surface",IJIRSET, Vol 3, Special Issue 4, April 2014.
- 11) Suresh Thokchom, Partha Ghosh and Somnath Ghosh, "Effect of water absorption, porosity and sorptivity on durability of geopolymer mortars". VOL. 4, NO. 7, SEPTEMBER 2009, ISSN 1819-6608
- 12) Hardjito, D., Wallah, S.E. and Rangan, B.V., *Study on Engineering Properties*, Fly Sh-Based Geopolymer *Concrete*. Journal of the Australasian Ceramic Society, 2002. 38(1): p.