



# 3<sup>rd</sup> ICTG 2016

04-07 September 2016, Guimarães, Portugal



University of Minho  
School of Engineering



# Modeling of lateral sleeper-ballast interaction on rail track

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UNIVERSIDADE NOVA DE LISBOA



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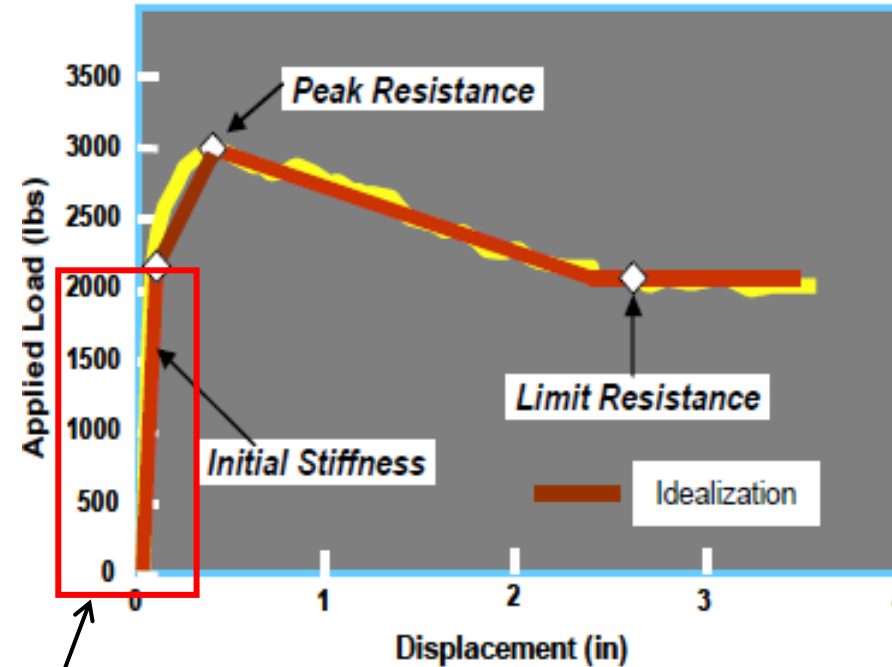
1. Introduction
2. Methods
3. Results
4. Conclusions
5. Further developments



## 1. Introduction

### Stiffness and lateral resistance of the track

Fig. 1- Lateral behaviour of the track. Kish (2011)



area under study



## 2. Methods

### Pegasus

*Developed by Varandas (2013)*

- Matlab program
- Linear (Hook's Law) and non-linear (K- $\theta$ ) ballast behaviour
- Non-linear contact between the sleepers and the ballast (Penalty formulation)

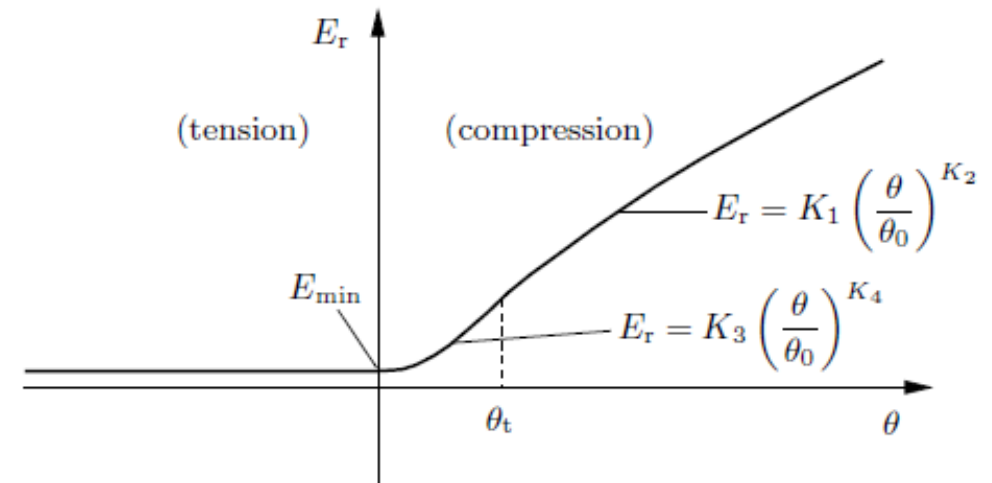


Fig.2 - The  $E_r$ - $\theta$  relationship . From Varandas (2013)



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## 2. Methods

### Model

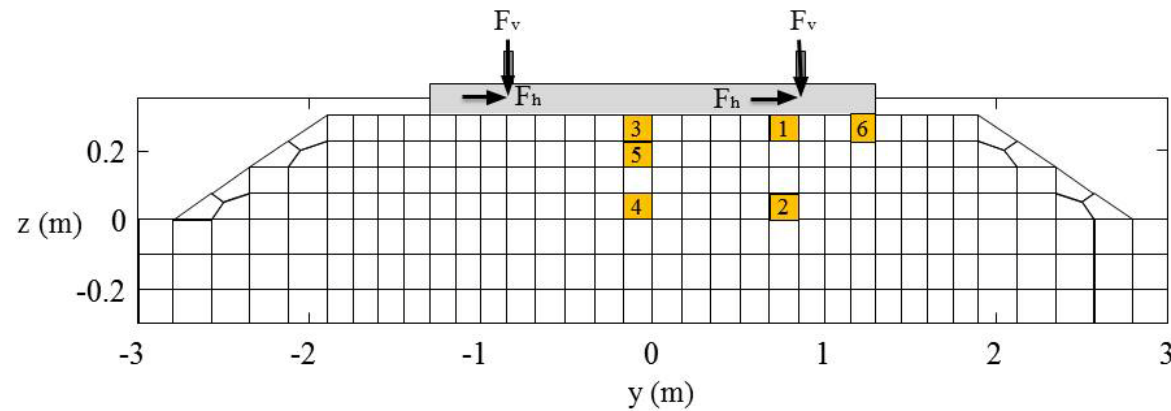


Fig.3 - Elements in study

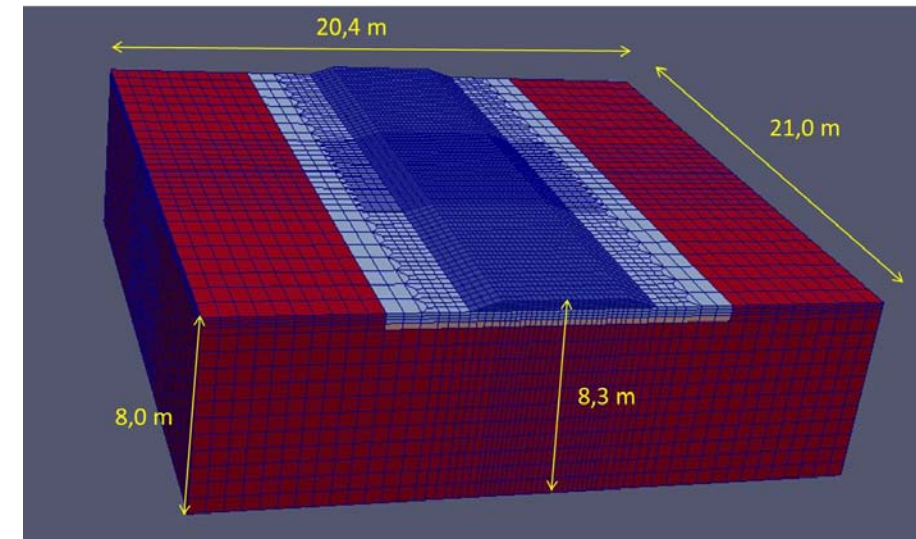


Fig.4- Geometry



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## 3. Results

### Vertical Load influence

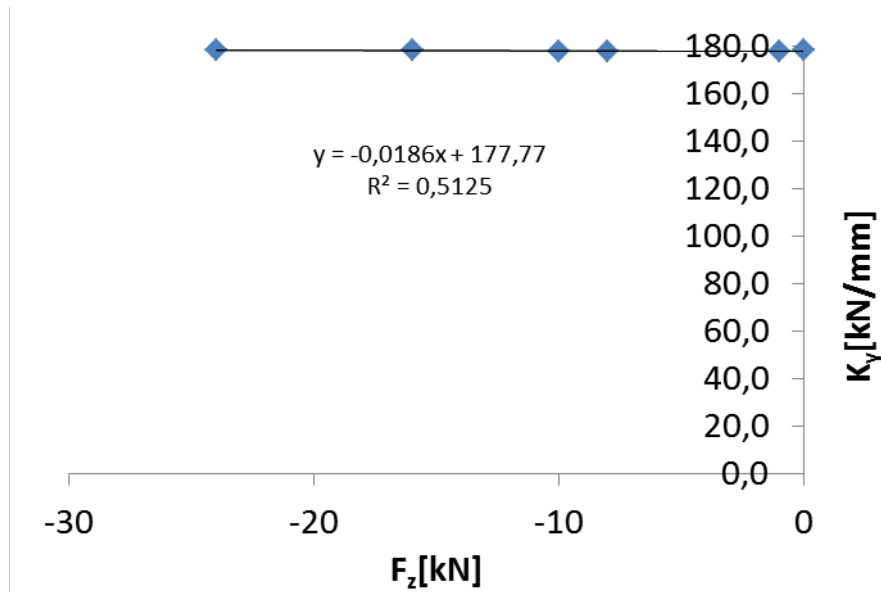
Test	Soil layer 1 :	Soil foundation :	$F_z$ [kN]	$F_y$ [kN]	$F_y/F_z$	$K_y$ [kN/mm]		$K_z$ [kN/mm]	
	$E$ [mPa] - $\nu$ - $\rho$ [t/m <sup>3</sup> ]	$E$ [mPa] - $\nu$ - $\rho$ [t/m <sup>3</sup> ]				Non Linear	Linear	Non Linear	Linear
D1	200 - 0,35 - 1,8	150 - 0,35 - 1,8	1	8	8,00	68,9	-	68,1	-
D2	200 - 0,35 - 1,8	150 - 0,35 - 1,8	0	8	inf	71,4	178,1	0,0	0,0
D3	200 - 0,35 - 1,8	150 - 0,35 - 1,8	-1	8	8,00	74,0	177,6	144,7	222,6
D4	200 - 0,35 - 1,8	150 - 0,35 - 1,8	-8	8	1,00	89,8	177,8	110,3	264,0
D5	200 - 0,35 - 1,8	150 - 0,35 - 1,8	-10	8	0,80	93,5	177,9	111,4	265,4
D6	200 - 0,35 - 1,8	150 - 0,35 - 1,8	-16	8	0,50	103,1	178,1	114,8	267,5
D7	200 - 0,35 - 1,8	150 - 0,35 - 1,8	-24	8	0,33	103,1	178,3	119,1	268,7
D8	200 - 0,35 - 1,8	150 - 0,35 - 1,8	-40	8	0,20	129,1	-	125,7	-
D10	200 - 0,35 - 1,8	150 - 0,35 - 1,8	-60	8	0,13	143,8	-	131,7	-
D11	200 - 0,35 - 1,8	150 - 0,35 - 1,8	-75	8	0,11	152,8	-	135,2	-
D12	200 - 0,35 - 1,8	150 - 0,35 - 1,8	-100	8	0,08	165,5	-	140,0	-



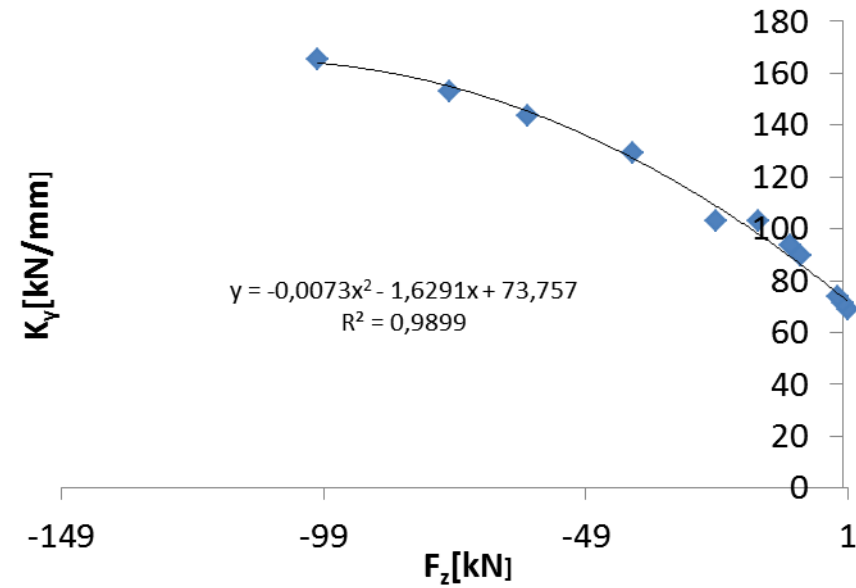
## 3. Results

Vertical Load influence – Stiffness :  $K_i = 2F_i / (u_{i,end} - u_{i,initial})$

Linear



Nonlinear

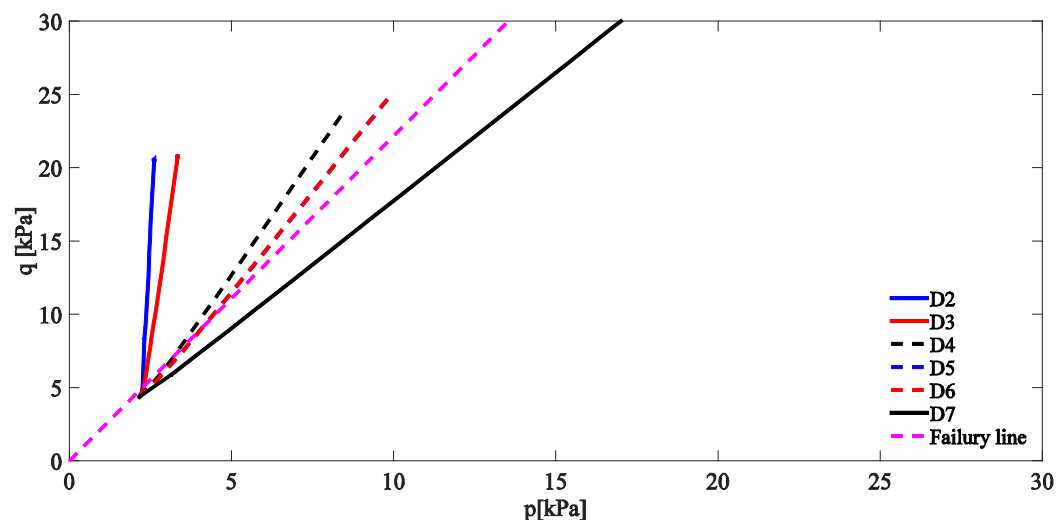




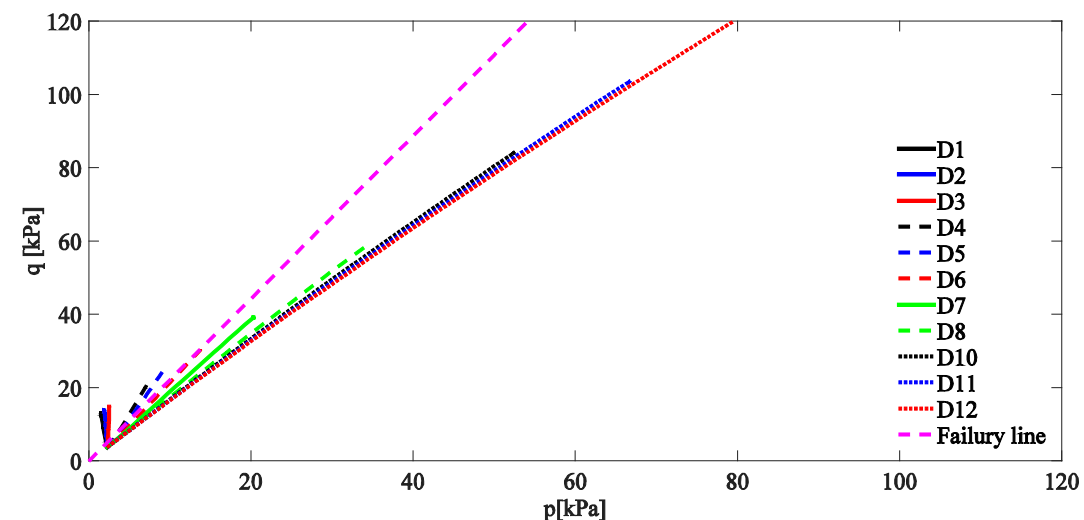
## 3. Results

### Vertical Load influence : p-q

Linear



Nonlinear







### Soil Foundation influence

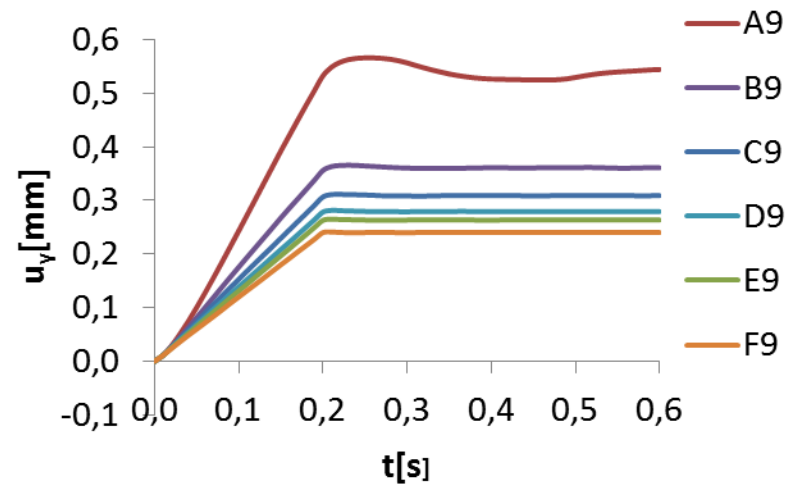
Test	Soil layer 1: E[mPa] - $\nu$ - $\rho[t/m^3]$	Soil foundation : E[mPa] - $\nu$ - $\rho[t/m^3]$	$F_z$ [kN]	$F_y$ [kN]	$K_y$ [kN/mm]		$K_z$ [kN/mm]	
					Non Linear	Linear	Non Linear	Linear
A9	200 - 0,35 - 1,8	20 - 0,45- 2	-75	25	83,6	161,7	110,2	227,2
B9	200 - 0,35 - 1,8	60 - 0,3 - 2	-75	25	121,9	91,8	169,0	105,4
C9	200 - 0,35 - 1,8	100 - 0,3 - 1,68	-75	25	140,1	189,7	232,8	292,8
D9	200 - 0,35 - 1,8	150-0,35-1,8	-75	25	152,0	208,2	269,5	341,9
E9	200 - 0,35 - 1,8	200 - 0,35 - 2	-75	25	159,6	138,5	301,4	164,7
F9	200 - 0,35 - 1,8	300-0,3-2,04	-75	25	169,7	178,9	338,2	265,9



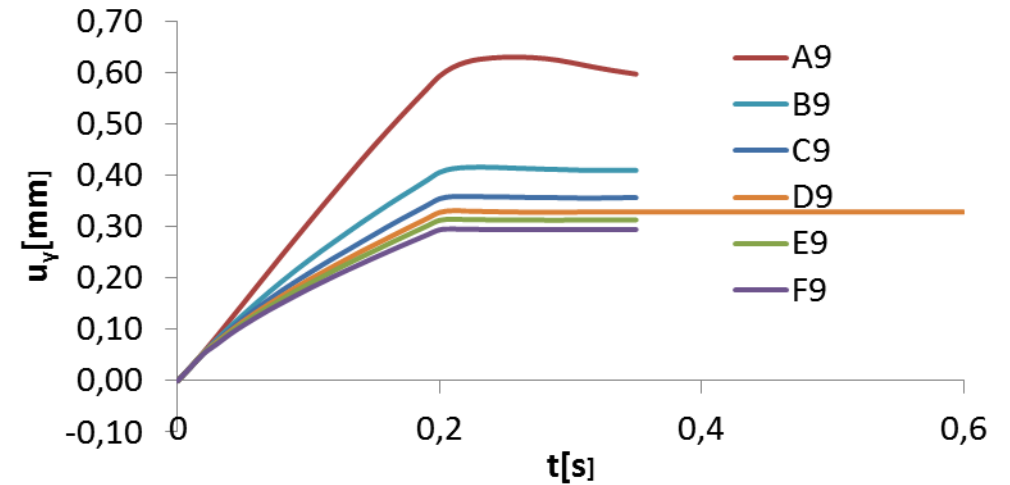
## 3. Results

### Soil Foundation influence : $u_y$

Linear



Nonlinear

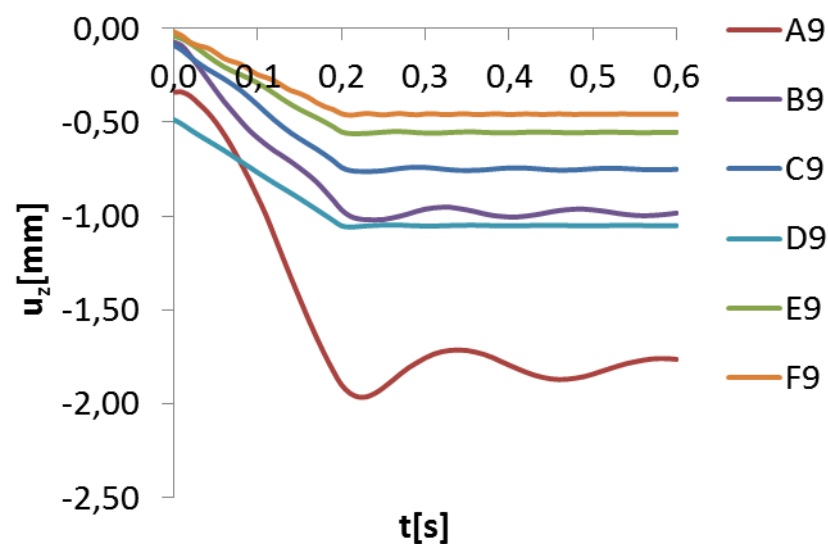




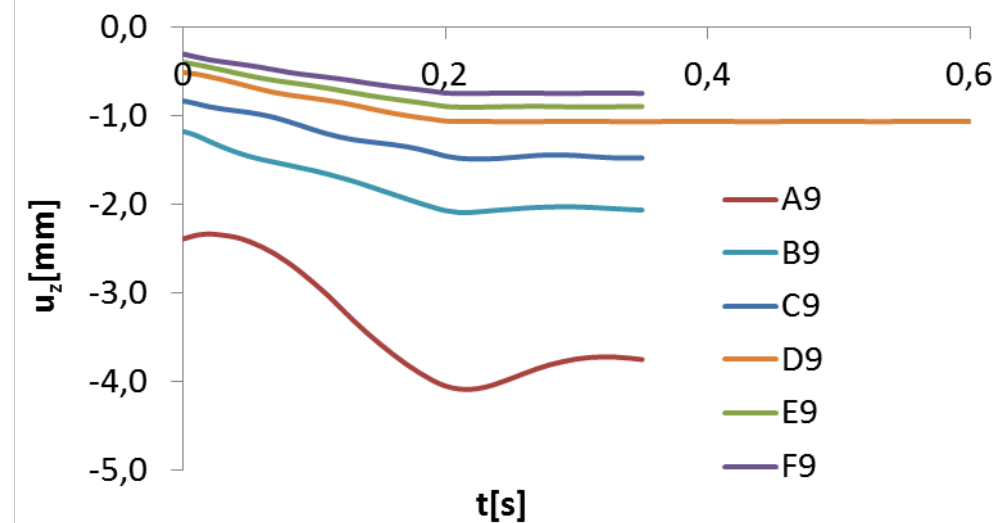
## 3. Results

### Soil Foundation influence : $u_z$

Linear



Nonlinear

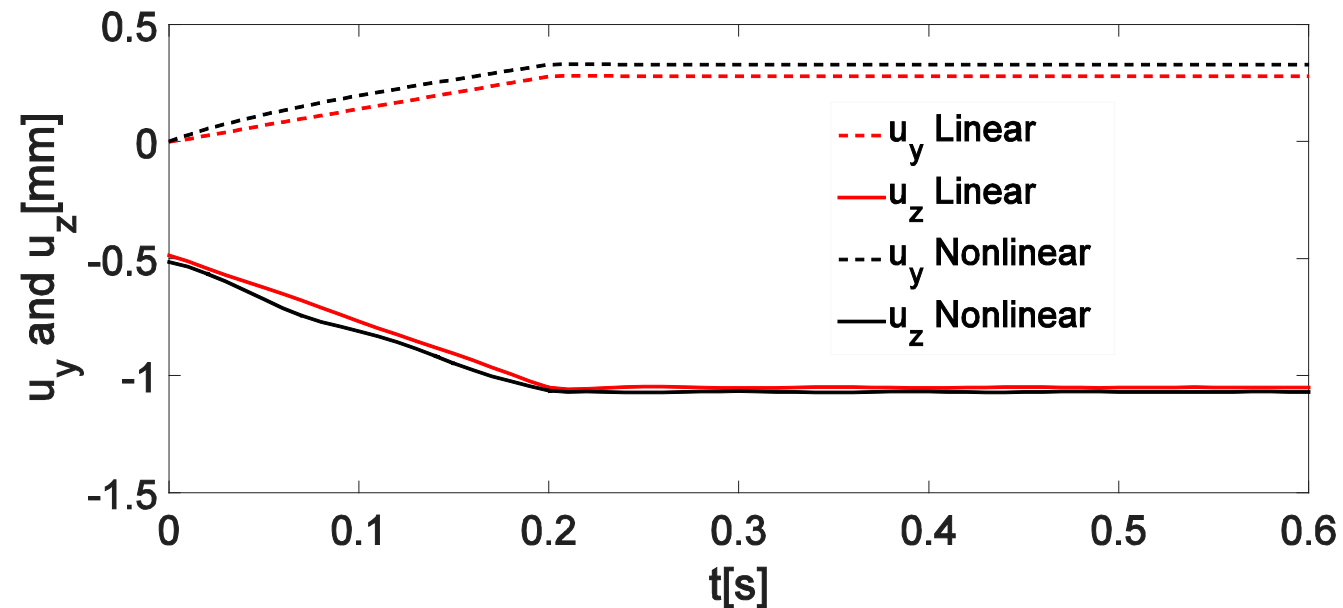




## 3. Results

Test D9 – comparison between the linear model and the nonlinear

Displacements

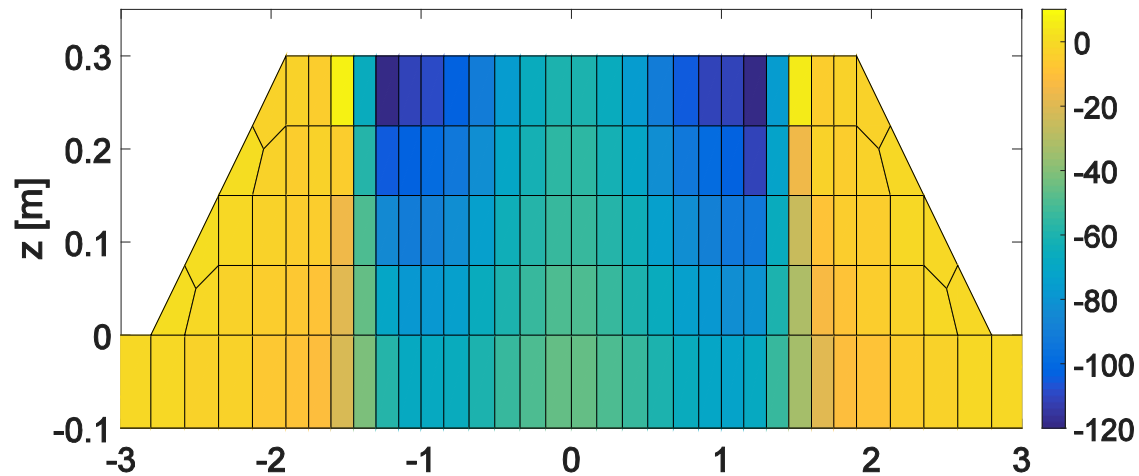




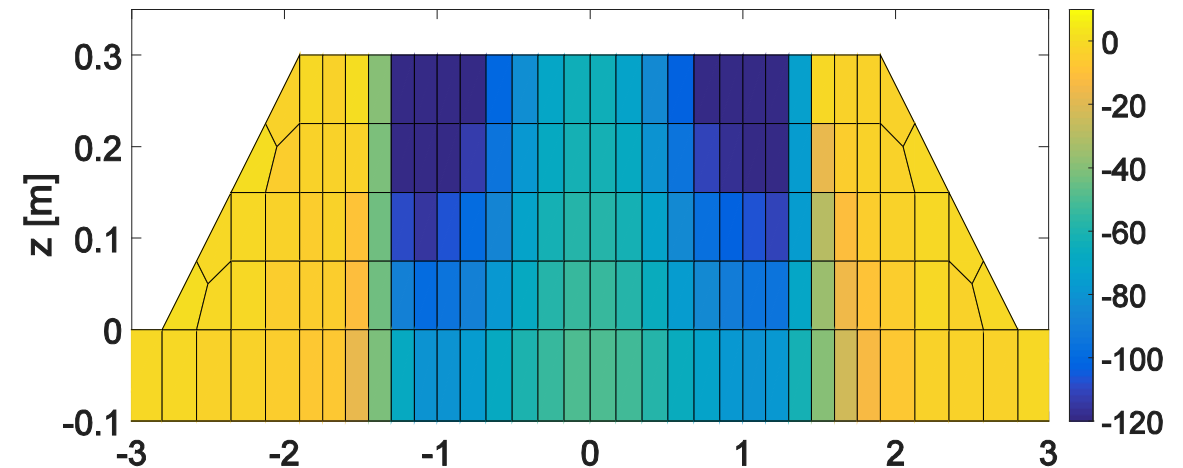
### Test D9 – comparison between the linear model and the nonlinear

Stress

$\sigma_z$  Linear



$\sigma_z$  Nonlinear





## 3. Results

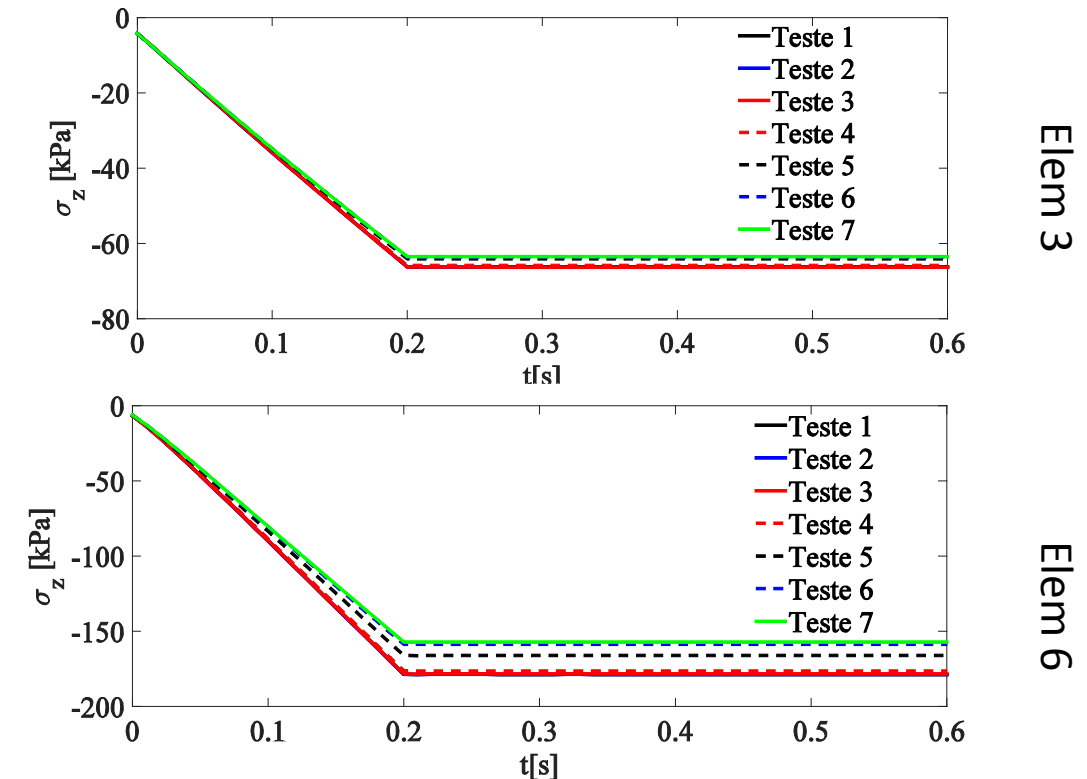
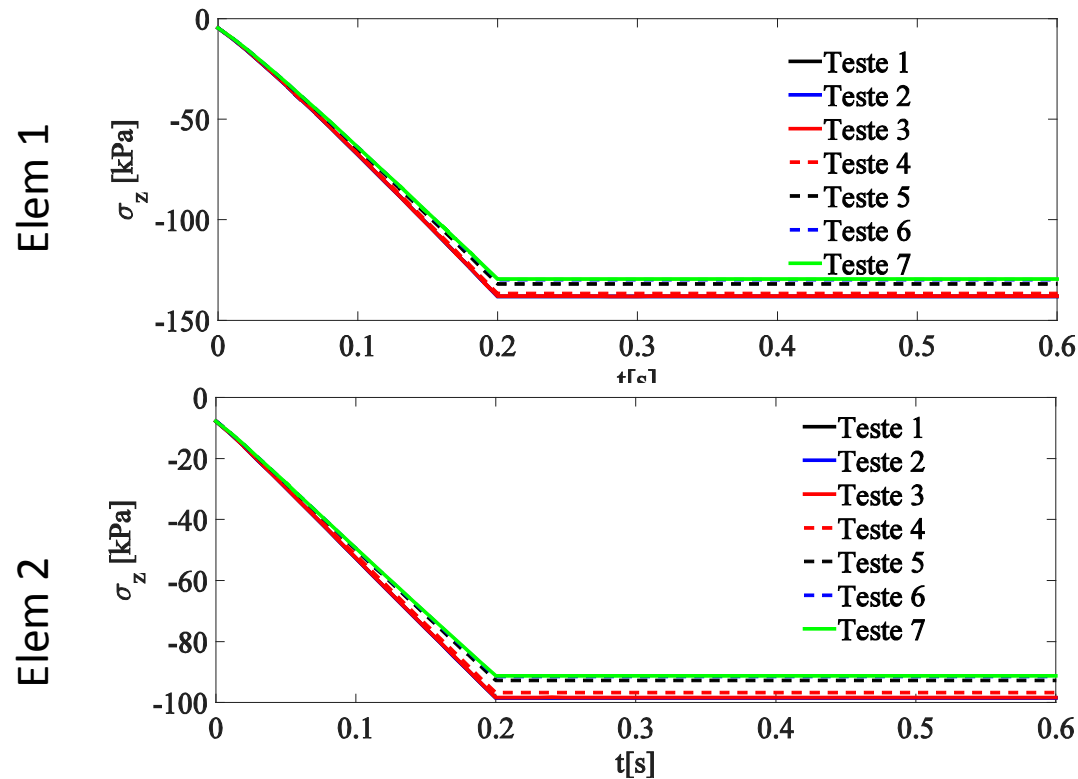
Influence of the lateral sleeper-ballast interaction ,  $K_{c,h}$  on the stress distribution in the ballast

	Test 1	Test 2	Test 3	Test 4	Test 5	Test 6	Test 7
$K_{c,h}[\text{kN/m}^2]$	0	$1 \times 10^2$	$1 \times 10^4$	$1 \times 10^5$	$1 \times 10^6$	$5 \times 10^6$	$1 \times 10^7$
$F_z[\text{kN}]$	-75	-75	-75	-75	-75	-75	-75
$F_y[\text{kN}]$	0	0	0	0	0	0	0
Foudation soil	D	D	D	D	D	D	D



## 3. Results

### Influence of the lateral sleeper-ballast interaction on the stress distribution in the ballast $-\sigma_z$

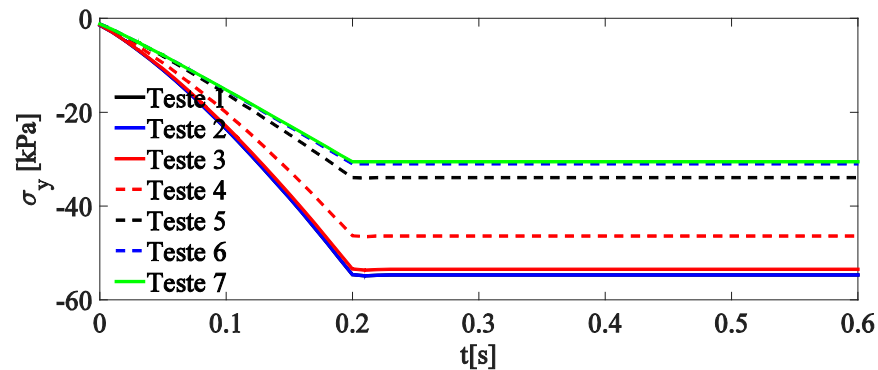




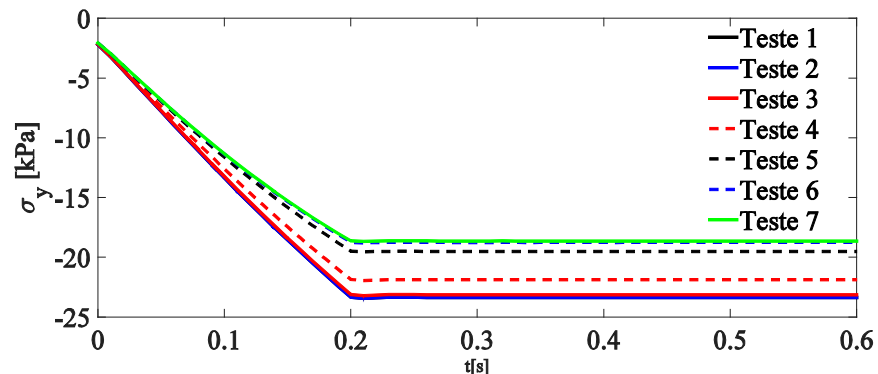
## 3. Results

### Influence of the lateral sleeper-ballast interaction on the stress distribution in the ballast - $\sigma_y$

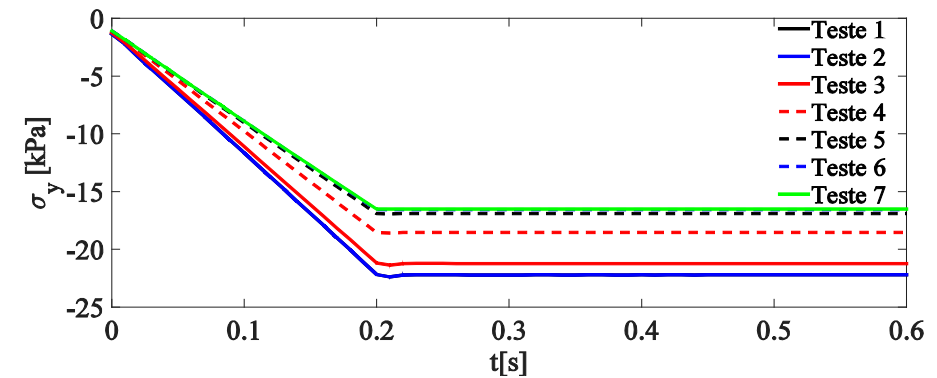
Elem 1



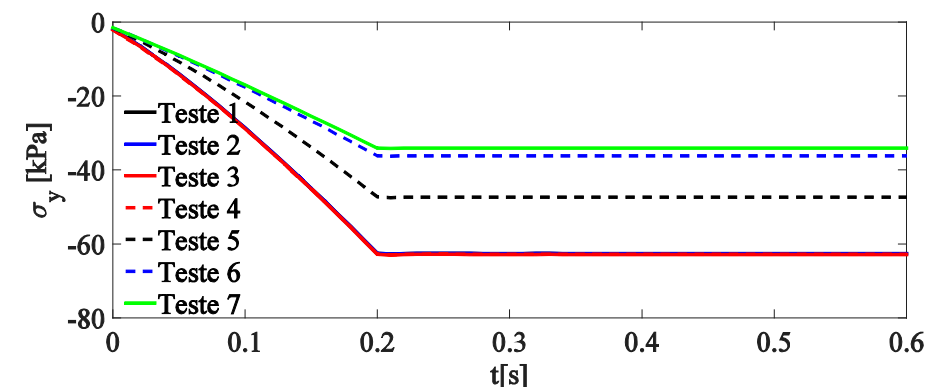
Elem 2



Elem 3



Elem 6







## Conclusions

- Load study – It was possible to obtain vertical load ( $F_z$ ) - lateral stiffness ( $K_y$ ) relation. For higher  $F_y/F_z$  relation, the graph p-q lies above the failure line .
- Soil foundation study – As expected, it was observed that the higher the Young modulus of the soil the higher the lateral stiffness .
- Influence of the lateral sleeper-ballast interaction on the stress distribution in the ballast – It is noted that the parameter  $K_{c,h}$  has a non negligible influence on the stress distribution inside the ballast layer, therefore denoting the importance of a care representation of this friction interface in studies focused on the granular layers of the track .



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# Thank you for your attention

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