



**3rd ICTG 2016**

04-07 September 2016, Guimarães, Portugal



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# Assessment of the short and long term behaviour of the track at a railway transition zone

Cristina Alves Ribeiro<sup>1</sup> e Rui Calçada<sup>1</sup>

1. *University of Porto, Faculty of Engineering, Portugal*





## What is a transition zone?

Zones where variation of vertical stiffness of the track occur. Those variations can be abrupt or smoothed by transition structures. Zones that require frequent maintenance operations.



Embankment - bridge



Culvert structures



Embankment - tunnel

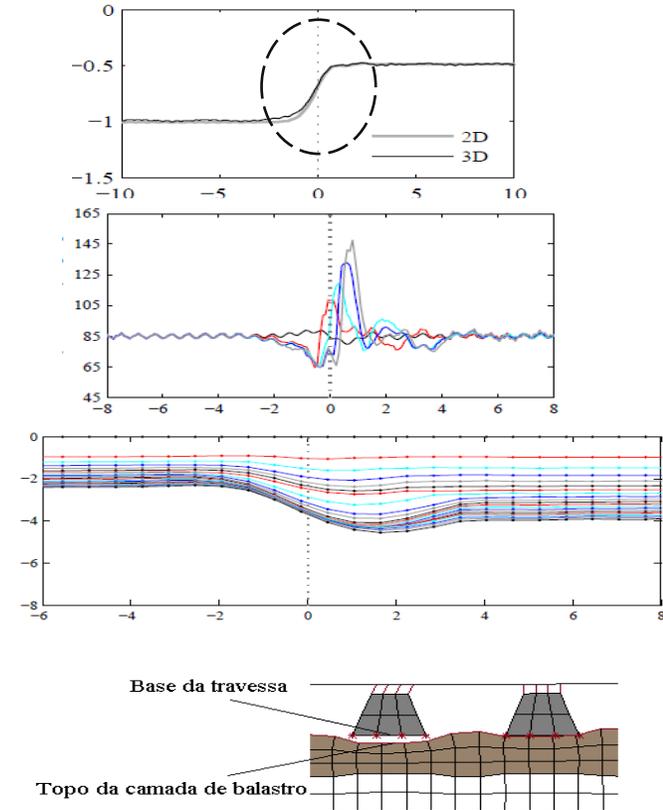


Ballast track – slab track



## Understand the problematic

<b>Variation of the vertical stiffness</b>	Assess how the stiffness variation can influences the dynamic behaviour.
<b>Variation of the wheel-rail contact forces</b>	Asses how those forces vary in short and long term.
<b>Differential settlements</b>	Understand the arise of differential settlement due to railway traffic – load repeatedly applied by trains
<b>Unsupported sleepers</b>	Understand the arise of unsupported sleepers: the sleeper is not working properly, there is an overload of the neighbour sleepers



### Background:

- Development of numerical models;
- Calibration and validation of the numerical models;
- Validation of the vehicle-track dynamics;



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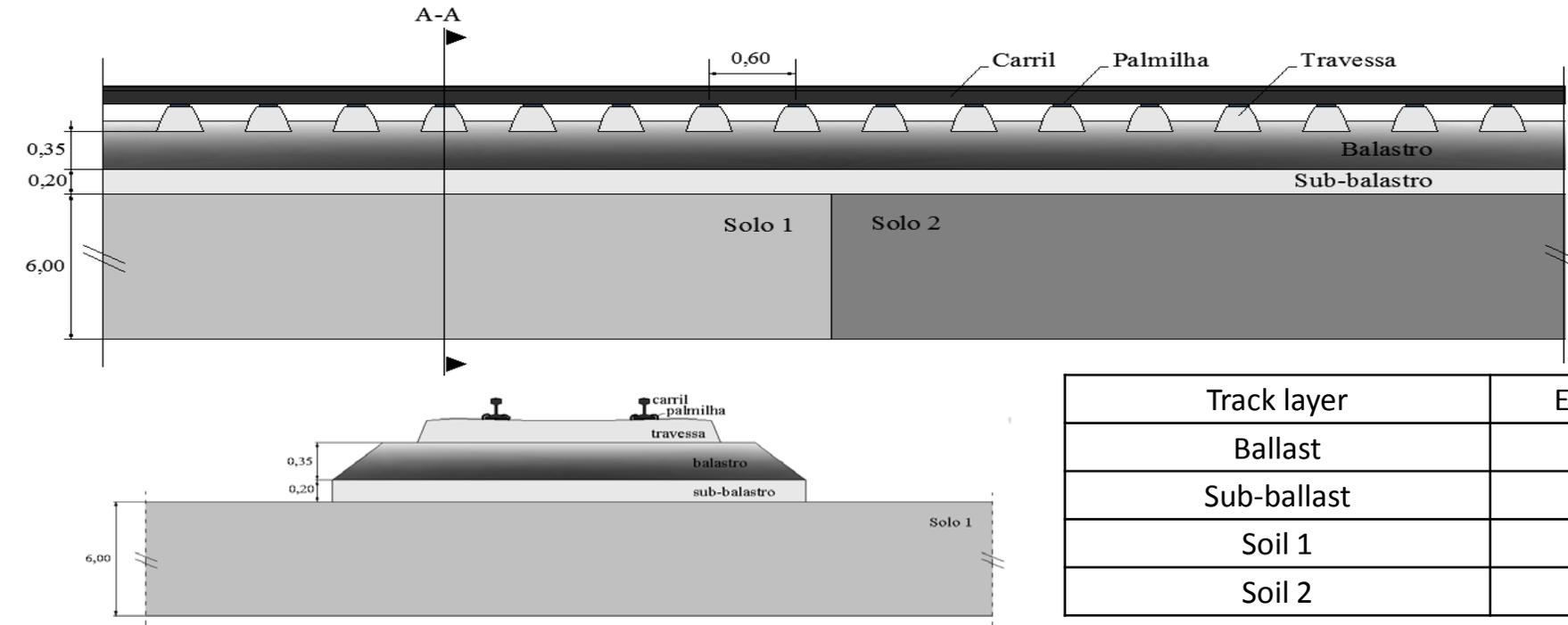
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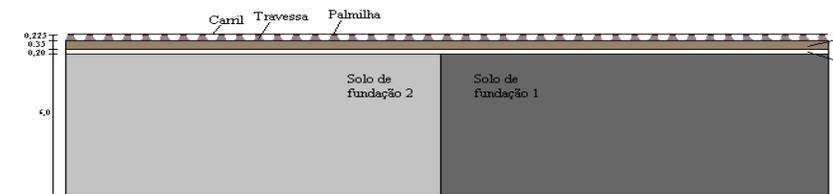
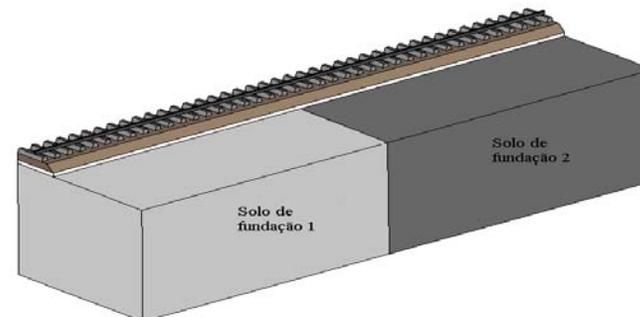
## Case study



Abrupt stiffness variation:  
Soil 1 → Soil 2

Track layer	E (MPa)	$\rho$ (kg/m <sup>3</sup> )	$\nu$ (-)
Ballast	130	1530	0,20
Sub-ballast	120	1935	0,30
Soil 1	80	2040	0,30
Soil 2	1600	2040	0,30

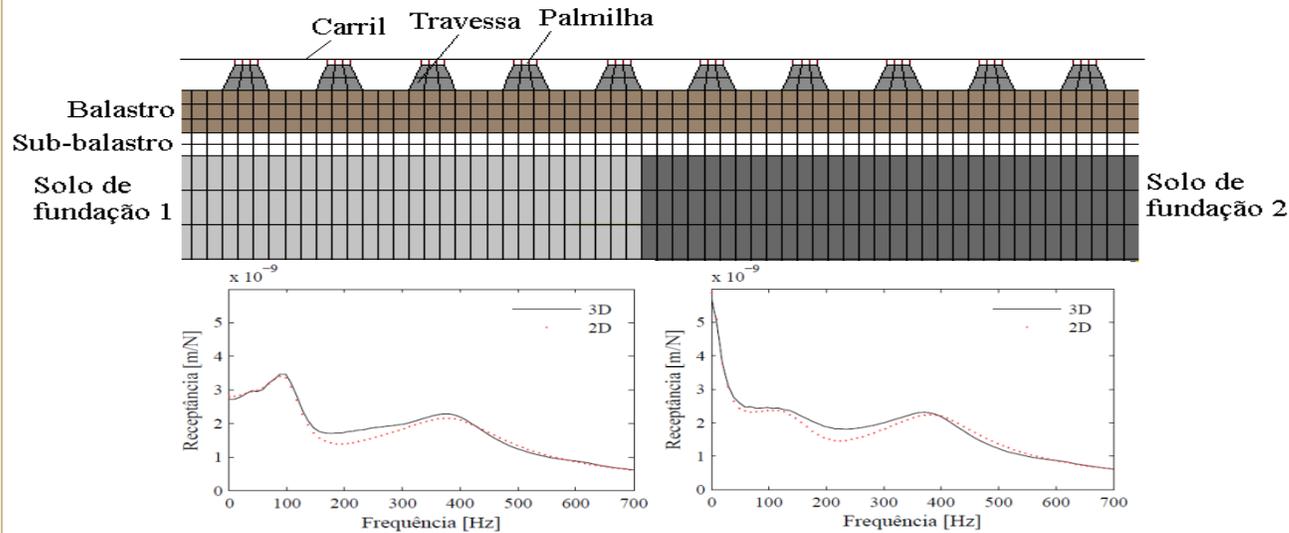
Numerical models:



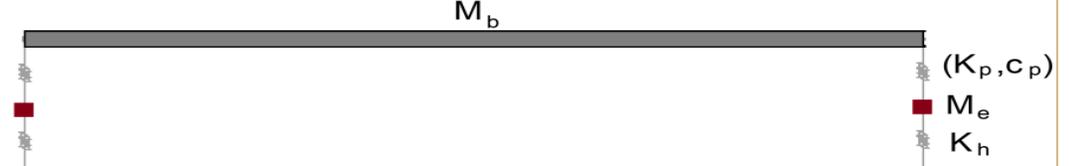


## Vehicle-track model

### TRACK – TRANSITION ZONE

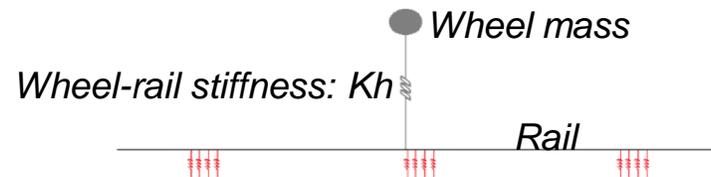


### VEHICLE – 1 BOGIE



Vehicle component		Value
Bogies	$M_b$	2200 kg
Primary suspension	$K_p$	$2,60 \times 10^6$ N/m
	$c_p$	$1,20 \times 10^4$ N/m
Wheels/axle	$M_e$	1700 kg
Wheel – rail stiffness	$K_h$	$1,35 \times 10^9$ N/m

### VEHICLE-TRACK INTERACTION

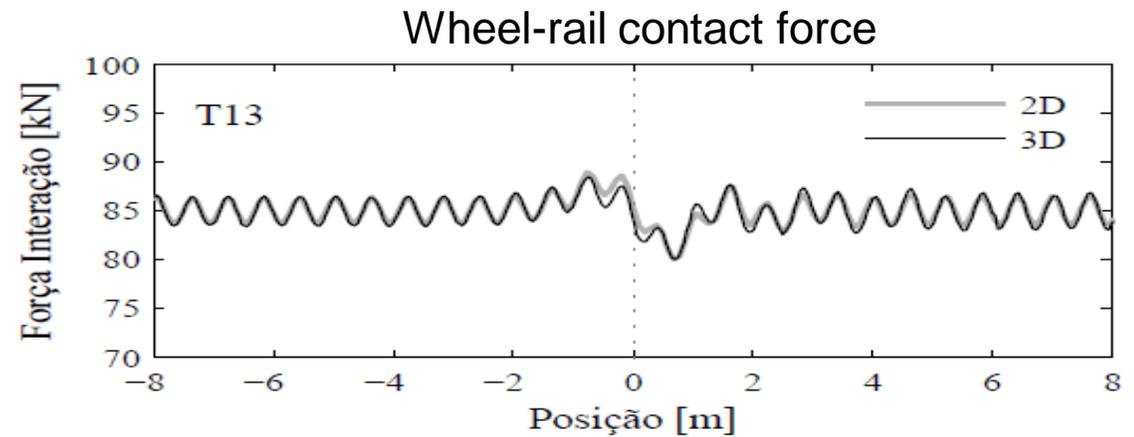
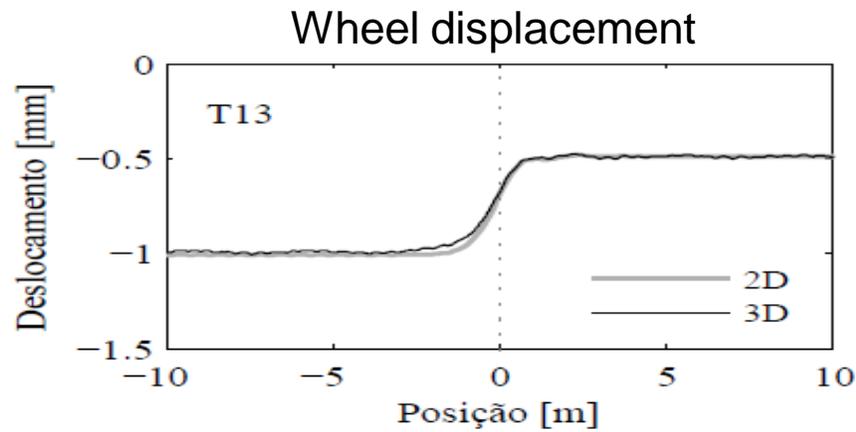


- Point-line contact
- Contact algorithms – ANSYS
- Penalty method

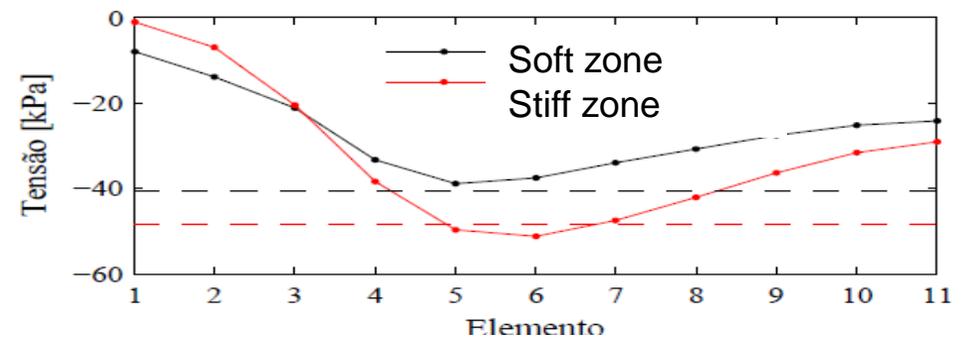
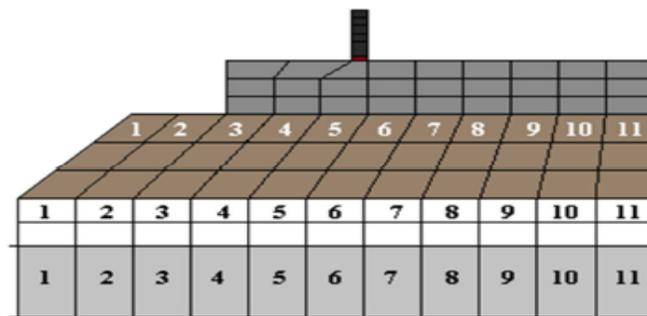


## Short term dynamic behaviour

Comparison of the results obtained in both 2D and 3D models

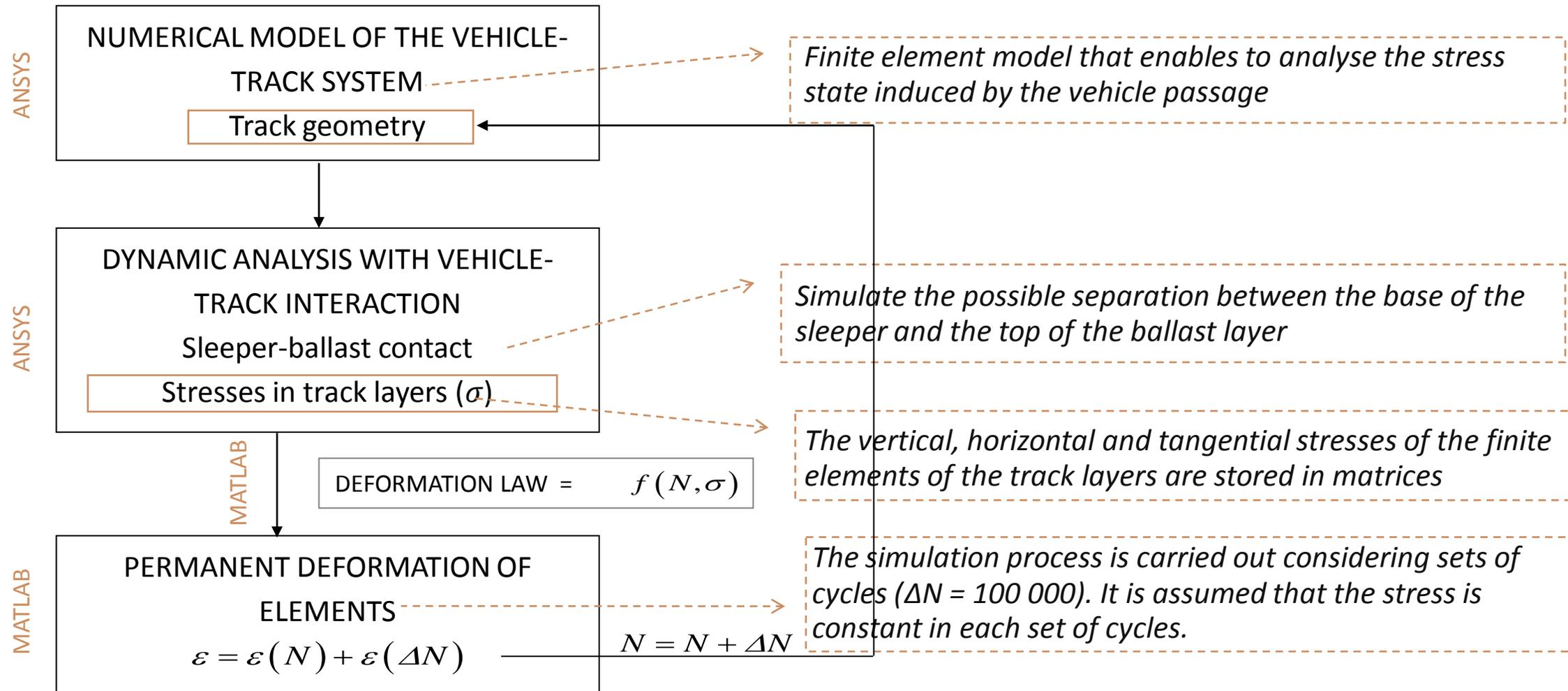


Maximum vertical stress (top of sub-ballast layer)



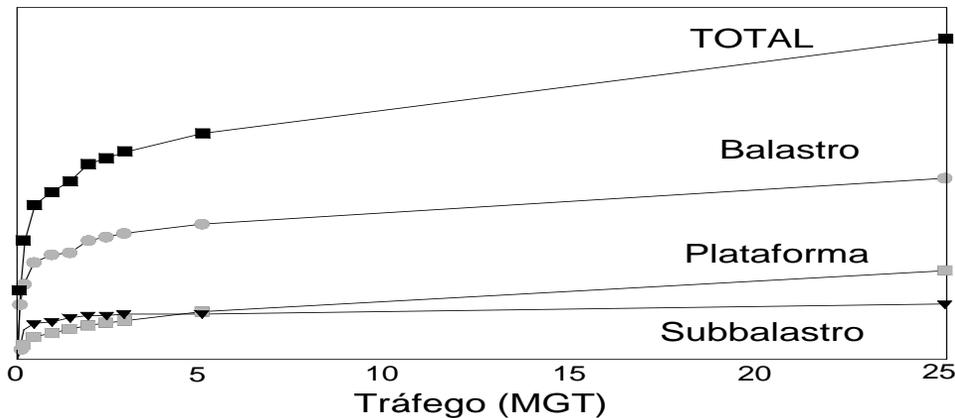


## How to simulate long term behaviour?





## Deformation laws



Permanent deformation of track layers

- Settlement growth is higher in ballast layer.
- Logarithmic laws.
- Depend on stress state and number of loading cycles

Ballast – ORE (1970) law

$$\varepsilon_N = \varepsilon_1 \left( C \log \left( \frac{N + N_i}{N_i} \right) \right)$$

Do not consider the phase 1 of loading

$$\varepsilon_1 = 0.082 (100n_p - 38.2) (\sigma_1 - \sigma_3)^2$$

Experimental validated by Ionescu (2004)

Depends on: N, stress, porosity of layer, constants

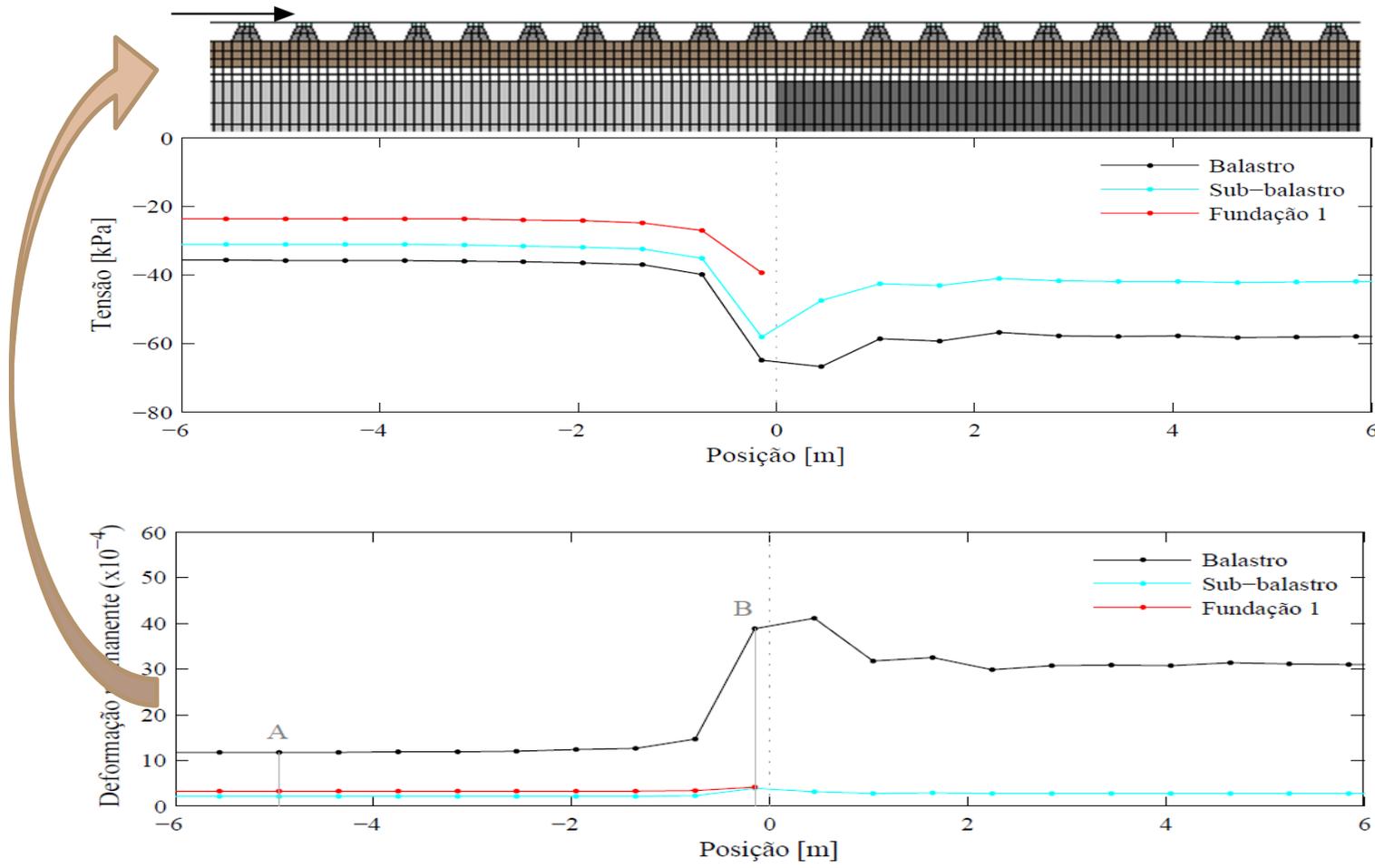
Sub-ballast; Foundation – Gidel et al. (2001):

$$\varepsilon_P = f(N) \cdot g(p_{\max}, q_{\max})$$

depends on: N, stress, constants that depends on the type of material



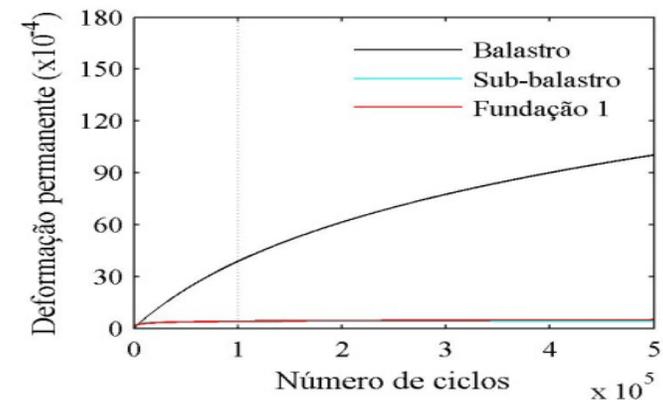
## Methodology for long term simulation



*Vehicle passage; dynamic analyses with wheel-rail interaction*

*Assess the stresses of the finite elements*

*Apply the deformation law of the materials for N cycles*

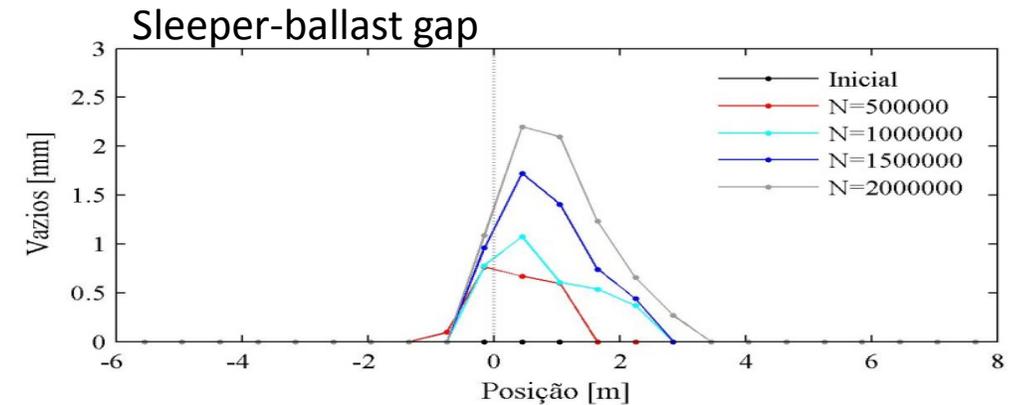
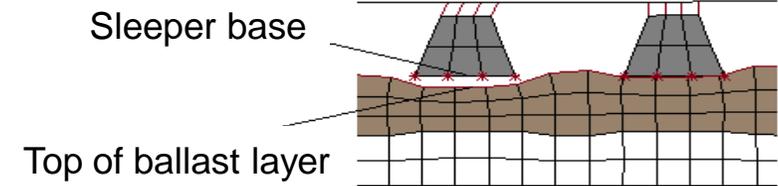
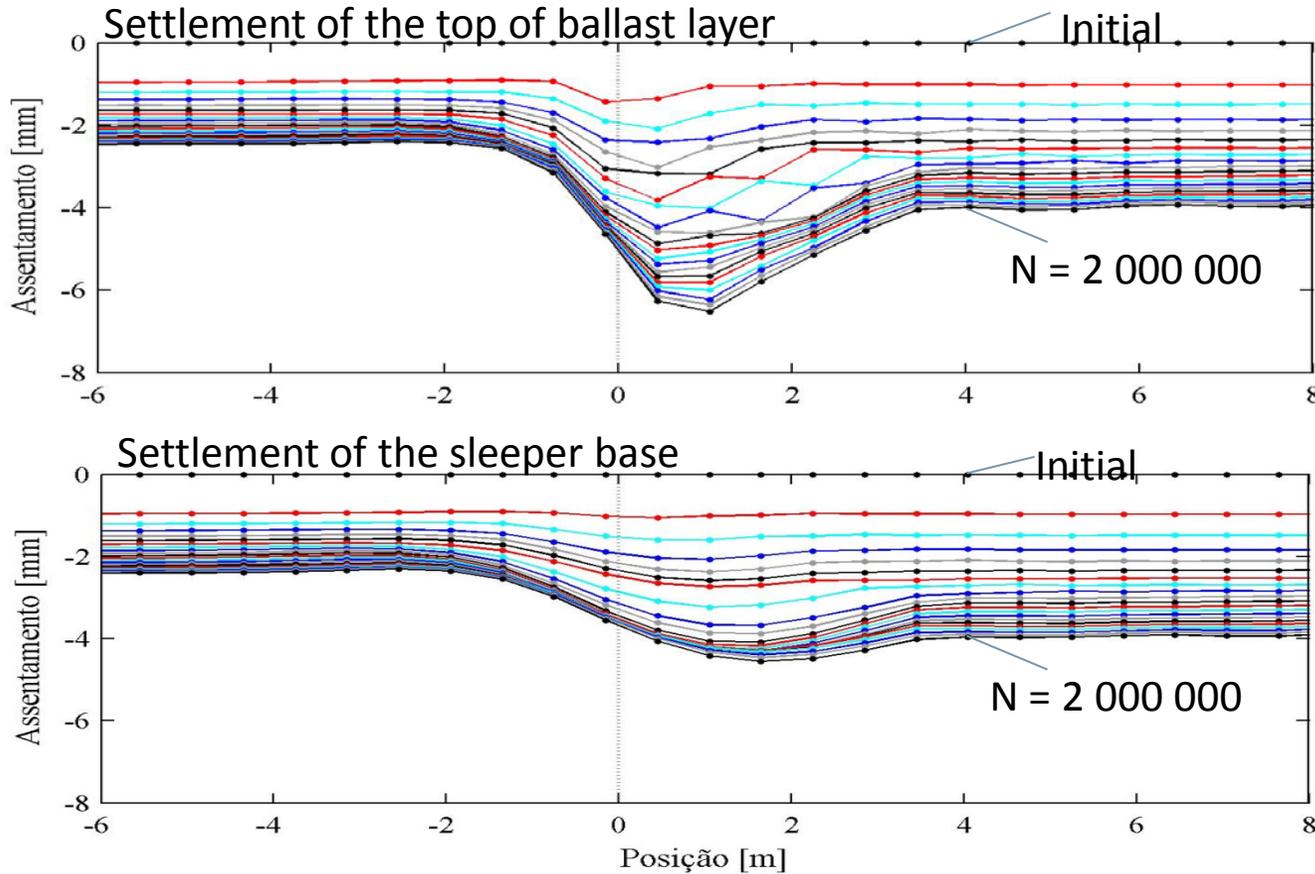


*Permanent deformation of the finite elements – update track geometry*



## Long term dynamic behaviour

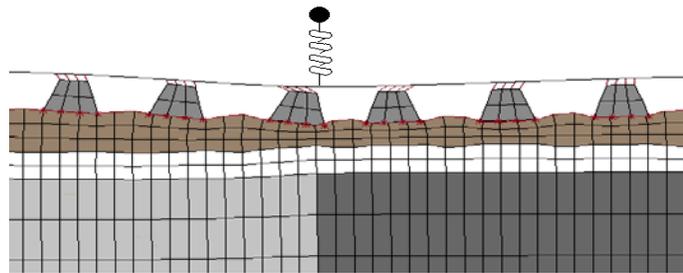
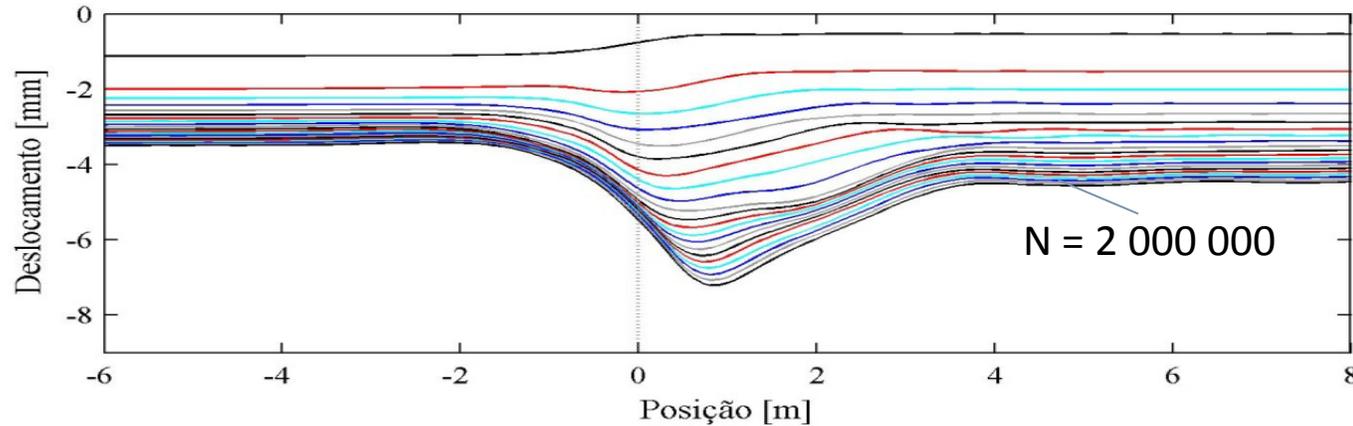
### Track settlement



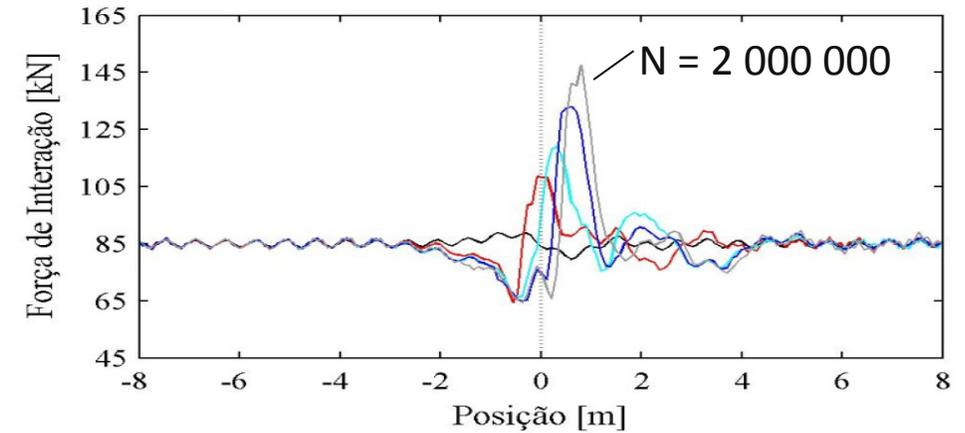


## Long term dynamic behaviour

### Vertical displacement of the vehicle wheel



### Wheel-rail contact force



Maximum amplification: 73,6%

It is very important to consider the dynamic component of the force on the simulation process

**600 000 cycles:** vertical acceleration of the axle is higher than **30 m/s<sup>2</sup>**  
**2 million cycles:** vertical acceleration reaches **70 m/s<sup>2</sup>** (immediate track correction)



## Conclusions

- I. Permanent deformation is higher when the deviatoric stress of the elements increase;
- II. The amplification of the dynamic loads that results from the track deformation, also contributes to its increase;
- III. The base of the sleepers do not follow the deformation of the track layers – gap appearance;
- IV. The ballast layer permanent deformation dominates the track deformation – both due to the law considered and the stress level installed on this layer;
- V. The dynamic effects obtained on the transition zone when permanent deformation is considered are higher than those obtained when there is only the stiffness variation.
- VI. This methodology can be applied to predict the long term behaviour of the track in other zones.



## Further developments

- I. Consider the track irregularities and track defects in the assess of the long term behaviour;
- II. Validate the results obtained for the long term behaviour with experimental data;
- III. Perform this analysis using different deformation laws;
- IV. Apply this methodology to assess the long term behaviour of other zones of the track.

**Recently it was created a user-interface platform and a user manual that enables anyone to use this application in models created in ANSYS program.**