

# Study of Seismic Effect on Different Types of Infill Walls

## Parth Shah, Roshni John



Abstract: The natural disaster that has occurred is an earthquake. Known to mankind for a very long time, researchers have been looking into various methods to protect the buildings ever since they were first discovered. Infill walls are one of the most significant components of the building sincethere was a need to limit the damage caused by it to the building, even though there are many different techniques to doing so. Infill walls are also efficient in providing stability to the building. Cladding can take the form of infill walls, which are constructed between the structural parts of a building. The structural structure offers support for the cladding system, while the cladding itself serves to partition the internal space from the outside world. Other types of cladding panels are attached to the exterior of the frame, but infill walls are installed in the spaces in between the framing components. This makes it distinct from other types of cladding panels. Although they are required to withstand wind loads imparted to the face, the infill walls that are being considered for this project are not being considered as load bearing infill walls. However, they are required to hold their own weight. In this project we will analysis the performance of a reinforced concrete building of regular plan with different kinds of infill walls using response spectrum method the structure is modelled with E-TABS

Keywords: Infill Walls, Response Spectrum Analysis, E-Tabs

#### I. INTRODUCTION

Infill walls are a type of cladding that can be found in buildings and are constructed into the spaces between the structural sections. The structural structure offers support for the cladding system, while the cladding itself serves to partition the internal space from the outside world. When compared to other types of cladding panel, infill walls stand out since it is attached between framing members rather than being affixed to the exterior of the frame. This makes it a unique option. A building that is constructed with a threedimensional framework structure will have a panelknown as wall that supported [kajal goel,2015,[2][4][5][6][7]].

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This panel completes the building's perimeter. Therefore, the structural frame is responsible for ensuring the bearing function, and the infill wall's role is to separate the inner space from the outer space while also filling in the boxes of the outer frames. The distinctive static function that the infill wall possesses is the ability to support its own weight. A sort of closure that is exterior and vertical in nature, the infill wall is opaque. The infill wall is distinct from other types of walls, such as the partition, which is utilized for the purpose of dividing upinternal space into two distinct areas.

#### II. AIM AND OBJECTIVES

#### Aim

The aim of the present study is to evaluate the response of infill walls subjected to seismic loads for regular plans located in zone 3 with medium soil condition. The response includes Story displacement, Story drift, Base shear, Time period.

#### B. **Objectives**

To compare following parameters for different irregular plan: a) Story displacement b) Story drift c) Maximum Story drift d) Base shear.

- To evaluate which infill wall is suitable for seismic forces.
- To analyses how particular infill wall behave differently for seismic analysis.
- To analyses the building in Seismic zone 3 with medium soil condition

#### III. LITERATURE REVIEW

The literature review includes an assortment of studies focused on the way of behaving of brick work infill walls inside different primary settings, especially under seismic circumstances. Fundamental examines a plan system for workmanship infill walls to upgrade their flexibility to inplane and out-of-plane burdens and stresses the significance of integrating infill walls into underlying modelcycles and assesses the effect of AAC block infill walls on primary elements [Adriano Reggle,2020, [1]]. Various sorts of infill wall materials and their conduct under consolidated stacking conditions. acquaints a creative development procedure with work on seismic execution by improving infill adaptability. An original brick work infill board development approach and its in-plane and out-of-plane reaction [Adriano Reggle,2020, [1]]. The prescient models for infill reaction in RC outlines. talks about the advantages of ILWFRC material for infill walls and investigates a development system zeroing in on infill itemizing for seismic execution.

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Accentuates the significance of involving reaction range strategies in seismic plan [IS:875 (Part 1) [3][8]]. The effect of infill walls on relocation and base shear in elevated structures. These examinations by and large add to understanding and working on the way of behaving of infill walls inside underlying frameworks, especially in seismic-inclined districts.

## IV. PROBLEM STATEMENT

Although in this particular plan we are considering different types of infill wall panels with open and closed story buildings and the plan has 9 bays in the x direction and 11 bays in the y direction, the building is considered to be in seismic zone 3 and soil type 2. The present study looks at an RCC structure with infill walls that is G+20 and studies the effect of seismic forces on it. The lateral loads that are going to be applied to the building are calculated according to the norms that are used in India. The study is carried out in order to conduct seismic analysis [Adriano Reggle,2020, [1]]. It will be designed using a software for the entire project in that for infill wall panel analysis equivalent strut method will be used in which the equivalent width W is given by W=0.175D( $\alpha$ h)0.4 is used after that we use response spectrum method after that we will look for results like Maximum displacement, Maximum drift, Base story, Time period response spectrum method. The beams and columns are adequately designed to withstand live loads and dead loads.

### V. METHODOLOGY

In this particular instance, the research is conducted on a G+20 story R C framed building with standard floor designs. The floor height is specified as 3 m, and the features of the framed construction are additionally defined. For Light weight concrete infill wall panel, the analysis is done for both closed and open story as well as Bare frame with shear wall. Ten models are carried out in software with different types of infill wall combination for closed and open story with Brick infill wall panels and Light weight concrete infill wall panels. When it comes to modeling, software such as e-tabs has been utilized with response spectrum method. The following types of models will be used for the project. Bare frame, Bare RCC frame with shear walls, Light weight concrete infill walls panel open story, Light weight concrete infill walls panel closed story, Brick masonry wall panels open story, Hollow Brick infill wall panel Timber infill wall panels, Precast infill wall panels, Aerated infill wall panels and Fly ash infill wall panels.

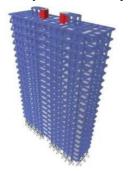


Fig. 1: Bare Frame

Fig. 2: ACC Blocks

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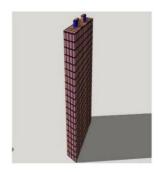


Fig. 3: Open Storey

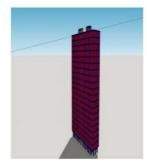


Fig. 4: Closed Storey



Fig. 5: Partial Shear walls Fig. 6: Brick Masonry





Fig. 7: Timber Infill walls Fig. 8: Precast Concrete Infill

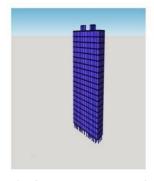




Fig. 9: Hollow concrete in fill Fig. 10: Fly-ash Brick infill

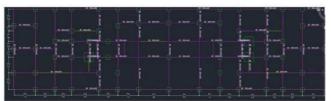


Fig. 11: Plan



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#### VI. RESULTS AND DISCUSSIONS

Following the completion of the response spectrum analysis, the parameters of the models of Light weight concrete infill wall panels with open and closed stories as well as Brick masonry infill wall panels for open stories are compared [kajal goe,2015, [2]] Theoutcomes are presented down below.

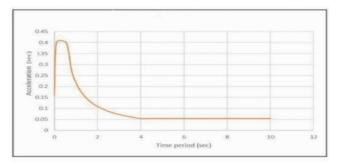


Fig. 12: Time and Acceleration Graph

### A. STORY DISPLACEMENT

Table No. 1: Story Displacement In X-Direction

Bare Fram e	Bare Frame with Partial Shear wall	Light Weight Concrete Infill Wall (Open Story)	Light Weight Concrete Infill Wall (Closed Story)	Brick Masonry Infill Wall
158.525	122.529	116.917	89.861	85.537
Hollow Concrete Infill Wall	Timber Infill Walls	Precast Masonry Infill Wall	Aerated Concrete Infill Wall	Fly Ash Brick Masonry Walls
122.85	111.315	79.805	115.05	111.388

# B. STORY DRIFT

Table No. 2: Story Drift in x-Direction

Bare Frame	Bare Frame with Partial Shear wall	Light Weight Concrete Infill Wall (Open Story)	Light Weight Concrete Infill Wall (Closed Story)	Brick Masonry Infill Wall
0.00596	0.002345	0.002992	0.002825	0.001808
Hollow Concrete Infill Wall	Timber Infill Walls	Precast Masonry Infill Wall	Aerated Concrete Infill Wall	Fly Ash Brick Masonry Walls
0.001957	0.003024	0.00170	0.00385	0.00205

- From the table 2 its observed that Precast masonry is the safest with 0.00170 mm (71.47%) lesser than Bare frame which also falls safe in .00170 mm which also is correct according to 0.004h formula (0.240 mm)
- From the above observations its seen that Bare frame without any infill walls is having drift up to 0.0596mm which is also the biggest.
- From Table No.1 we can see that Story displacement is maximum for Bare frame with no infill walls or shear walls which is 158.98 mm which fails to fulfill the requirement of h/500.
- The safest building which has come is Precast concrete

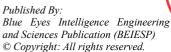
- building which is most preferable in terms of Story displacement that is 79.08 mm (50.25%) lesser than Bare frame.
- After Precast Brick masonry infill panel is the safest wall in terms of Story displacement that is 85.56 mm (46.18%) followed by Light weight infill panel closed story 89.86mm (43.47%) lesser than Bare frame.
- After Bare frame the second lowest is Bare frame with shear walls i.e., 115.05mm (27.63%) lesser than Bare frame which fulfills the requirement of h/500 which is lesser than it.
- Rest other infill walls are also falling in the parameter of the value less than h/500, so they are also safe Light weight infill panel open story116.917mm (27%), Hollow Brick infill walls 122.85(22.71%), Timber infill walls 111.315mm (29.96%) and Aerated concreteinfill 115.08 mm (29%) lesser than Bare frame.
- For Light weight concrete infill wall (open story) 0.002992 mm (46.18%), for closed story 0.002825 mm (50.13%), forBrick masonry infill walls 0.001808mm (67.48%), Bare frame with shear wall 0.002345 mm (57.82%), for Hollow concrete Brick infill wall its 0.001957 mm (64.80%), Timber infill walls its0.003024mm (45.61%), fly ash Brick wall infill its 0.00205 mm (63.12%) lesser than Bare frame.
- All the values fall under the criteria of 0.004h so the frames are safe but Precast is more preferable frame according to Story drift in x-direction observations.

#### C. BASE SHEAR

Table No. 3: Base shear in x-Direction for Response Spectrum Method (kN)

Bare Frame	Bare Frame with Partial Shear wall	Light Weight Concrete Infill Wall (Open Story)	Light Weight Concrete Infill Wall (Closed Story)	Brick Masonry Infill Wall
4752	9467.04	9276.96	9313.92	12318.24
Hollow Concrete Infill Wall	Timber Infill Walls	Precast Masonry Infill Wall	Aerated Concrete Infill Wall	Fly Ash Brick Masonry Walls
8896.80	8820.70	15131.04	7440.576	10417.44

- Precast infill wall has the highest Base shear 15131.04 kN followed by Brick masonry 12318.24KN and they both also have the least displacement and drift.
- Bare frame without wall has the least Base shear 4752
- kN and it also had the most displacement.
- Light weight concrete (closed story )9276.96 kN, open story 9313.92 kN, Bare frame with partial shear wall 9467.04 kN, Hollow concrete infill walls 8896.8 kN and for Timber infill walls 8820.7 kN
- For fly ash Brick masonry 10417.44 kN and Aerated concrete infill walls its 7440.576 kN





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#### D. TIME PERIOD

Table No. 4: Time Period

Bare Frame	Bare Frame with Partial Shear wall	Light Weight Concrete Infill Wall (Open Story)	Light Weight Concrete Infill Wall (Closed Story)	Brick Masonry Infill Wall
4.321	2.1366	2.655	2.659	0.8485
Hollow	Timber	Precast	Aerated	Fly
Concrete	Infill	Masonry	Concrete	As
Infill Wall	Walls	Infill Wall	Infill Wall	hBrick
				Masonry
				Walls
2.433	1.458	0.755	3.25	1.37

- From the above table Precast concrete infill wall has the least Time period0.755 sec (82.52%) followed by Brick masonry 0.8485 sec (80.36%) lesser than Bare frame.
- For Timber infill walls the Time period remained
- 1.458 sec (66.25%) and for fly ash Brick masonry it is around 1.37 sec (68.29%) lesser than Bare frame.
- Bare frame with partial shear walls the Time period is 2.1366 sec (50.55%) and for (Light weigh concrete closed story) and for (Light weight concrete open story) it is 2.655 sec (38.32%) &2.659. sec (38.46%) lesser than Bare frame.
- For Aerated concrete the Time period taken is 3.25 sec (24.75%) lesser than Bare frame and for Bare frame without walls the Time period taken is 4.321.
- From the above analysis it shows that Precast masonry is the safest in terms of Time period and Bare frame without any walls is the least safe.

## VII. CONCLUSIONS

In the current research study, a basic try analyzes to study thebehavior and pattern of seismic. waves of buildings with various types of infill walls in this case G+20 RCC framed building is used and 10 models are analyzed open story Light weight concrete infill panel, closed story Light weight concrete infill, open story Brick masonry and the observations made are as follows:

- Story displacement for Light weight concrete (open story) in X-direction is 116.87 mm for Light weight closed story is 89.861 mm, for Brick masonry its
- 85.537 mm, for Bare RCC frame with partial shear wall its 126.037 mm, for Hollow Brick infill wall its
- 122.85 mm, Timber infill wall its 111.315 mm, for Precast masonry its 79.805 mm, for Aerated concrete block infill wall its 115.05 mm, Fly-ash Brick walls its 111.388 mm which is into the permissible limits h\400 and the least is for Precast concrete masonry walls & followed by Brick masonry infill wall panel, observed that Precast masonry is the safest with0.00170 mm (71.47%) lesser than Bare frame which also falls safe in .00170 mm which also is correct according to 0.004h formula (0.240 mm).
- Story displacement for Light weight concrete (open story) in Y-direction is 37.174 mm, for Light weight concrete (closed story) is 36.152 mm, Brick masonry is

34.532 mm and for Bare frame with partial shear wall its 39.56 mm, for Hollow Brick infill 35.05 mm, Timber infill wall 30.001 mm, Precast masonry infill walls 23.605 mm, Aerated concrete. Bricks infill walls 45.605 mm, Fly ash Brick infill walls 31.601 mm and for Bare frame without any walls its 54 mm the values are within the permissible limits h\400 and the minimum is for Precast concrete masonry infill wall panel. In the Story displacement (y direction) the Precast concrete (56.665%) lesser than Bare frame.

- Story drift for Light weight concrete (open story) in X-direction is within the permissible limit 0.004 h and the minimum are for Precast masonry 0.00170mm (71.47%) lesser than Bare frame infill.
- Story drift for Light weight concrete (open story) in Y-direction is 0.002061 mm, for Light weight concrete (closed story) is 00.01734 mm, Brick masonry is 0.00089 mm and Bare. frame with partial shear wall its 0.00136 mm, Hollow concrete infill walls 0.00209 mm, Precast masonry infill walls 0.00090 mm, Aerated concrete infill walls 0.0195 mm, fly ash concrete infill walls 0.00121 mm, Bare frame 0.00335 mm the values are within the permissible limit 0.004h and the minimum is for Brick masonry infill wall panel. So, in Story drift the Precast wall masonry0.00090 mm (73.13%) lesser than Bare frame is better than other infill wall panels followed by Brick masonry.
- Base shear for X-direction Light weight concrete infill wall (open story) is 9276.96 kN for closed story its 9313.92 kN, Brick masonry infill walls its 12318.24 kN, Bare frame with partial shear wall its 9467.04 kN, for Hollow concrete infill wall sits 8896.80 kN for Timber infill walls its 8820.70 kN for Precast masonry its 15131.04 kN which is the most as its Story displacement also came less Aerated concrete infill walls its 7440.576 kN fly ash. Brick masonry its 10417.44 kN and Bare frame its 4752 kN which is the least.
- Base shear for Y-direction Light weight concrete infill wall (open story) is 3962.035 kN for closed story its 3977.823 kN, Brick masonry infill walls its 5325.072 kN, Bare frame with partial shear wall its 4092.52 kN, for Hollow concrete infill wall sits 3849.75 kN for Timber infill walls its 3767.20 KN for Precast masonry its 6462.21 kN which is the most as its Story displacement also came less Aerated concrete infill walls its 3216.499 kN fly ash Brick masonry its 4449.012 kN and Bare frame its 2039.5 kN which is the least as its Story displacement also came the most.
- Base shear for x direction came the most for Precast concrete infill walls and the least was of Bare frame.
- Base shear in y direction also showed the same trends of that in x -direction the Story displacement for the Precast concrete was the least and that for the Bare frame was the most so the checks are correct.
- From the above graph Precast concrete infill wall has the least Time period0.

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- 755 sec (82.52%) lesser than Bare frame, and which is the least and the safest among all.
- Precast masonry infill walls are the best suited for earthquake resistance design followed by Brick masonry.

#### **FUTURE SCOPE**

- 1. In this study the work is done using a regular plan further the work will be extended using irregular plans like L-shaped, T-shaped plans.
- The study can also be continued using different seismic zones, soil type, and different parameters will be studied. The study can also be carried forward using plans of more no of floors.
- 3. Further study can also be carried out using time history method.
- 4. For further study the analysis using wind load analysis can be done.

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Ethical Approval and Consent to Participate	No, the article does not require ethical approval and consent to participate with evidence.	
Availability of Data and Material	Not relevant.	
Authors Contributions	All authors have equal participation in this article.	

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#### **AUTHORS PROFILE**



Parth Shah, a postgraduate, is pursuing Masters of Technology, in the branch of Structural Engineering from Department of Civil Engineering, Saraswati College of Engineering, Mumbai University and currently in his final year. Research area is Earthquake Engineering and also working on site in designing field.



**Prof. Roshni John,** currently working as a Professor, Department of Civil Engineering, Saraswati College of Engineering, Mumbai University and currently in his final year. Her Major Area of research is earthquake engineering, she has vast experience in teaching fielf and abundance knowledge in structural field.

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