

On the impact of operational uncertainties on water distribution system design

Preliminary insights on Anytown

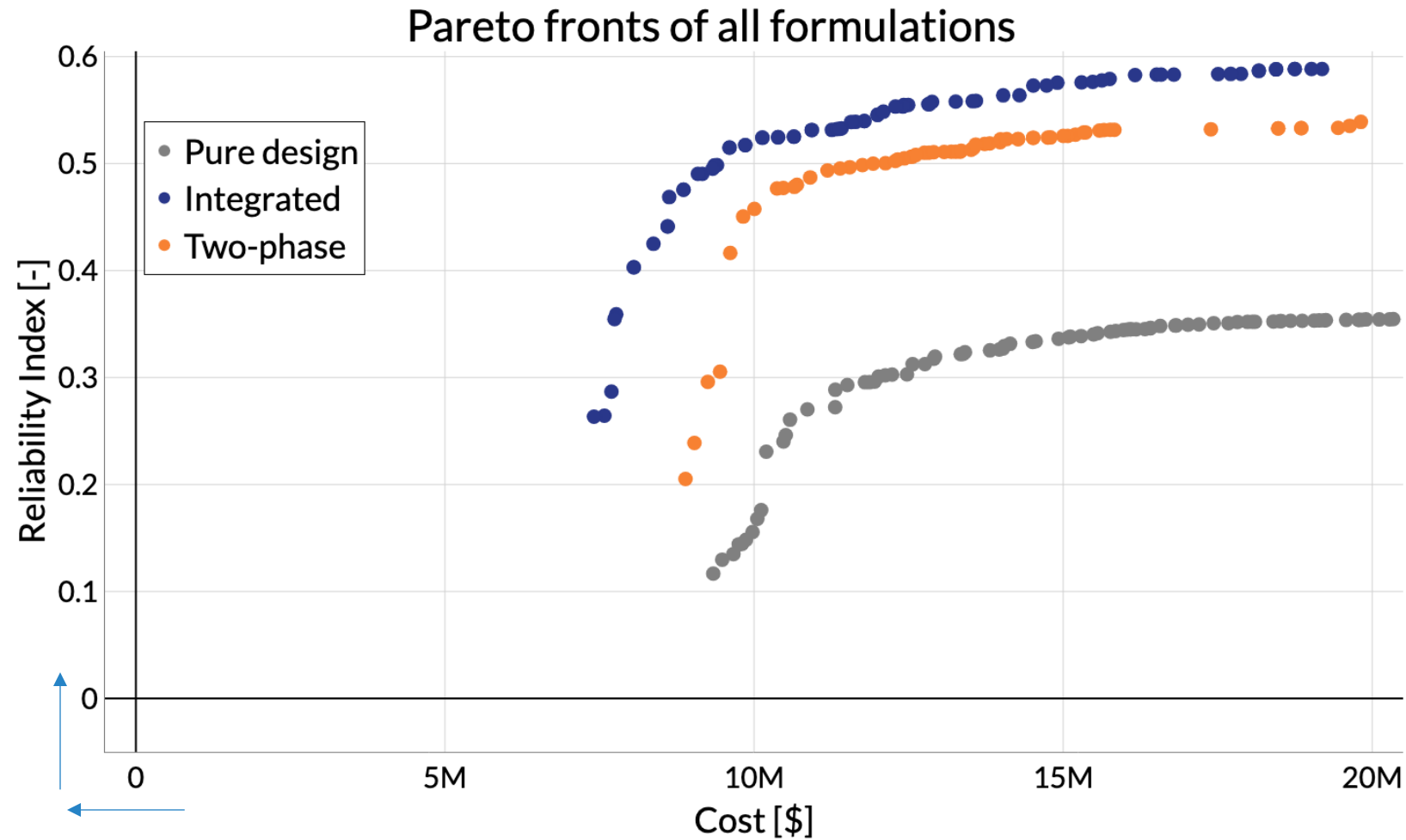
Dennis Zanutto, 2nd year Ph.D candidate
Prof. Andrea Castelletti,
Prof. Dragan Savić

KWR



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Uncertainty or Flexibility?



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Research Objectives and Motivation

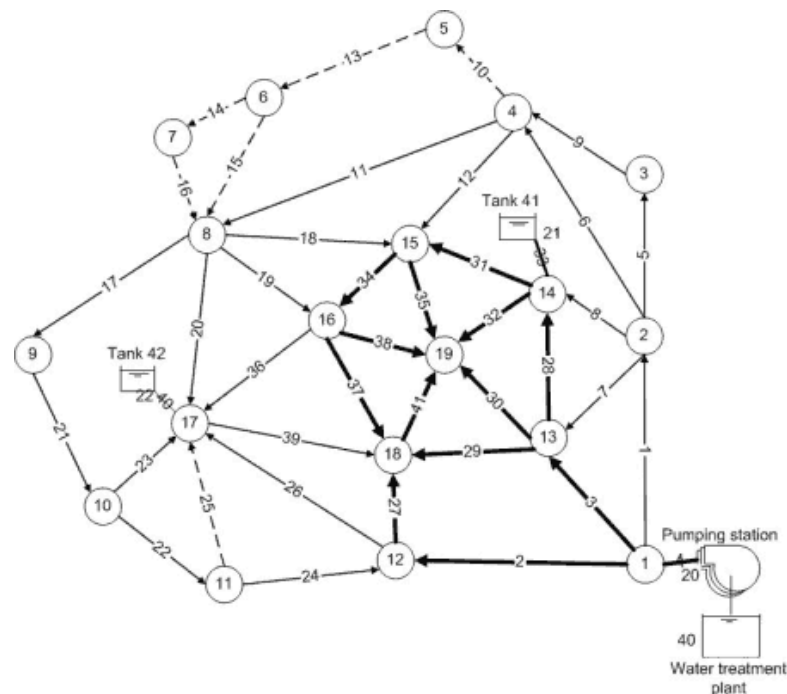
Long-term PhD research question:

Can we **introduce flexibility in WDS planning**/long-term design **through the operations** of the active elements?

Steps:

1. Why "operational flexibility"?
2. Does this hold in other contexts?
3. **How do we introduce operational flexibility in WDS planning?**
 1. **How sensitive are WDSs to operations?** *Today's focus!*
 2. How do we quantify operational flexibility?
 3. How do we exploit operational flexibility?

Case Study: Anytown*



Objectives

Net Present Value (Cost)

C_0 Immediate capital expenditure

C_t Yearly pumps' electricity cost

$$NPC = C_0 + \sum_{t=1}^{25} \frac{C_t}{(1+r)^t}$$

Todini's Reliability Index

A measure of energy surplus to meet required demand q^* and head h^* . [Todini, 2000](#)

$$I_r = \frac{\sum_{i=1}^{n_j} q_i^* (h_i - h_i^*)}{\sum_{k=1}^{n_s} Q_k H_k + \sum_{j=1}^{n_p} P_j / \gamma - \sum_{i=1}^{n_j} q_i^* h_i^*}$$

Decision variables

Design

- | | | |
|------------|----------------|------------|
| New pipes | Existing pipes | New tanks |
| - Diameter | - Do nothing | - Location |
| | - Clean | - Volume |
| | - Duplicate | |

Operations

- Pumping station
- Schedule

Constraints (Design requirements)

Meet demand and min. pressure at

- instantaneous peak flow
- fire flow conditions
- average daily pattern



*Full original formulation: [Walski et al., 1987](#)

Methodology

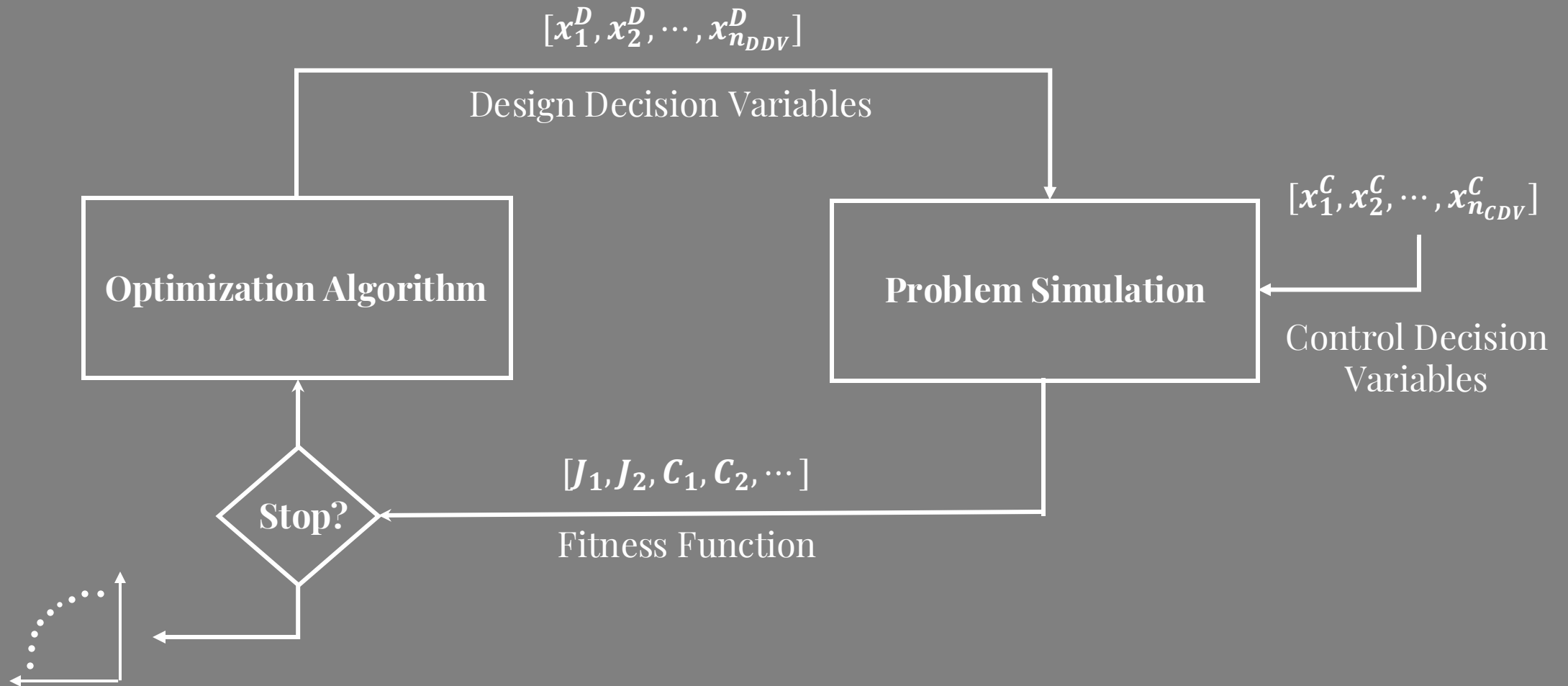


Pure design

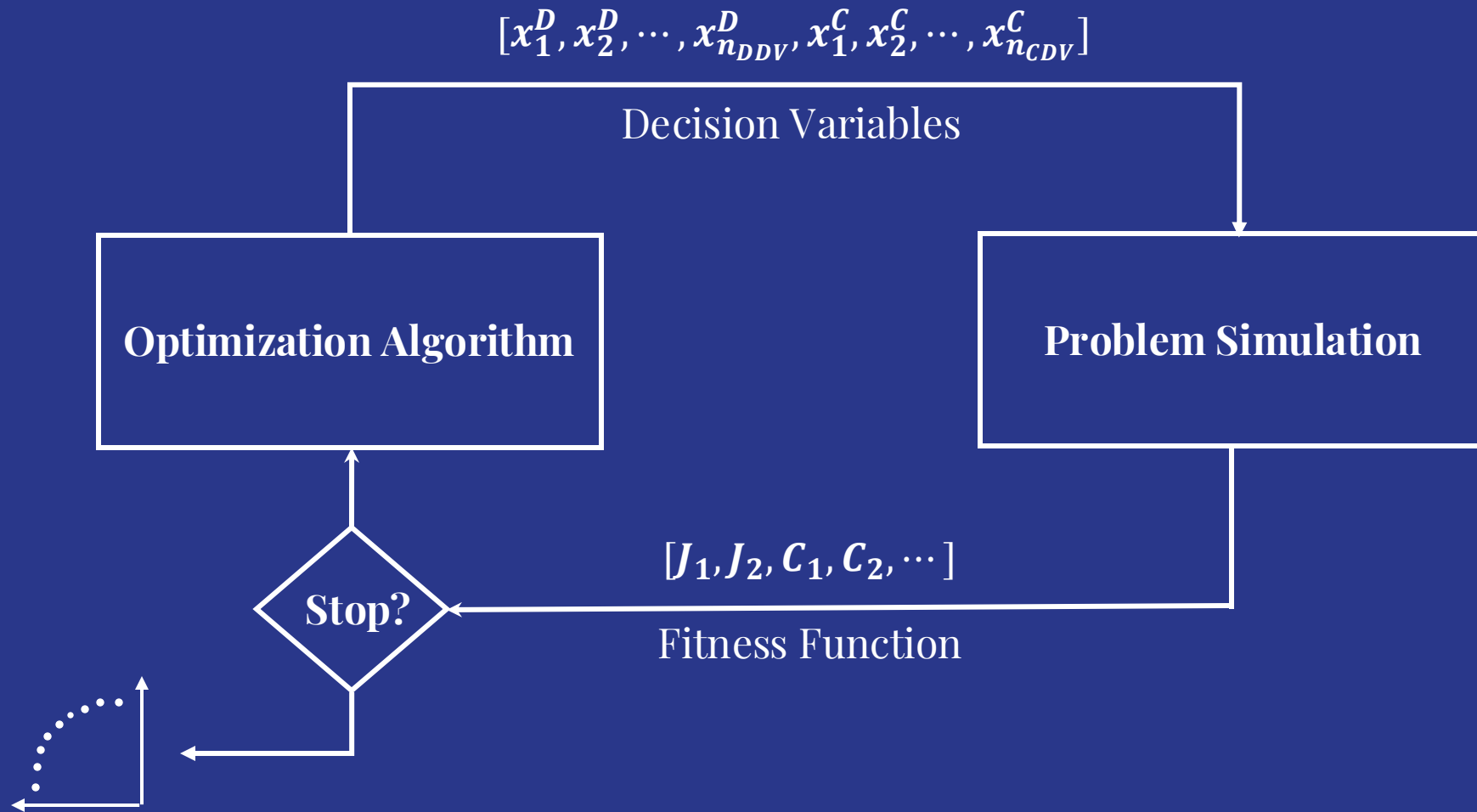
Integrated

Two-phase looped

Pure design

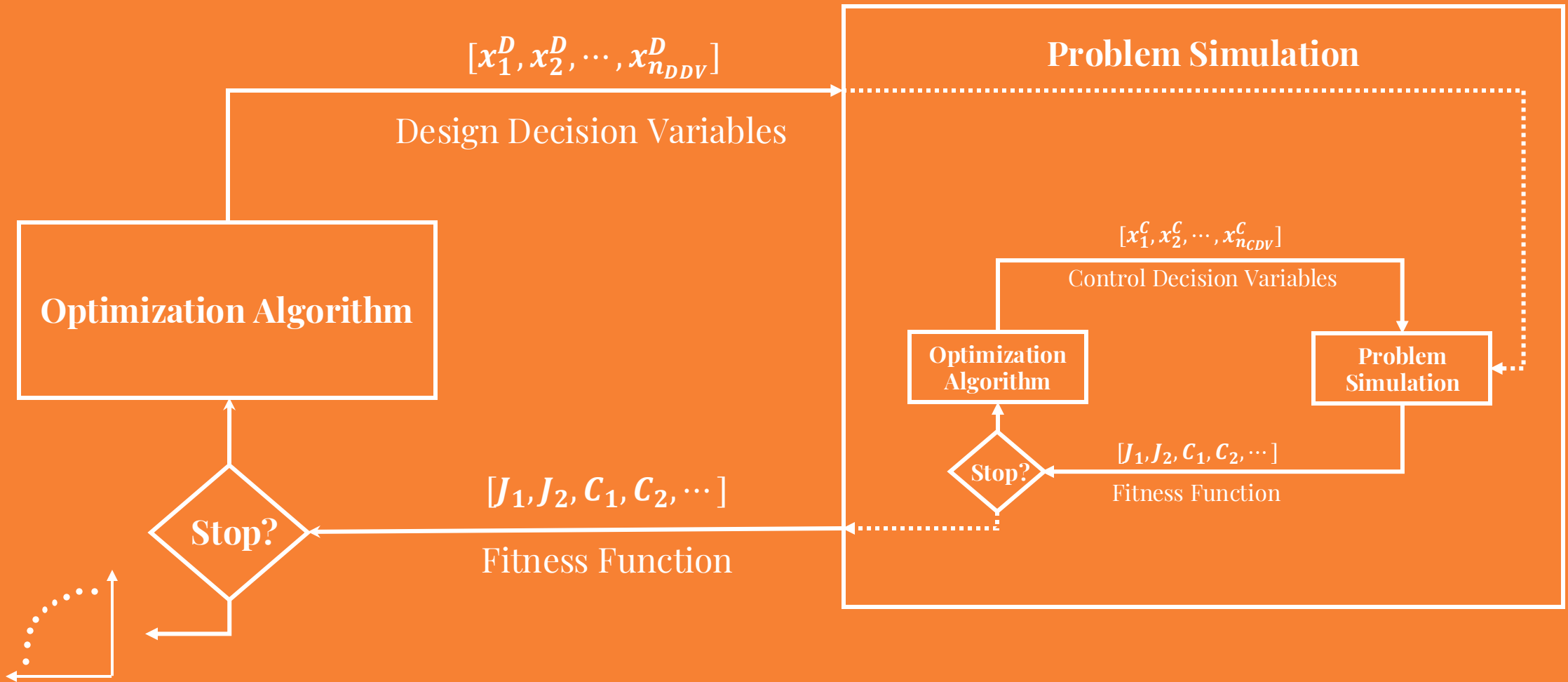


Integrated

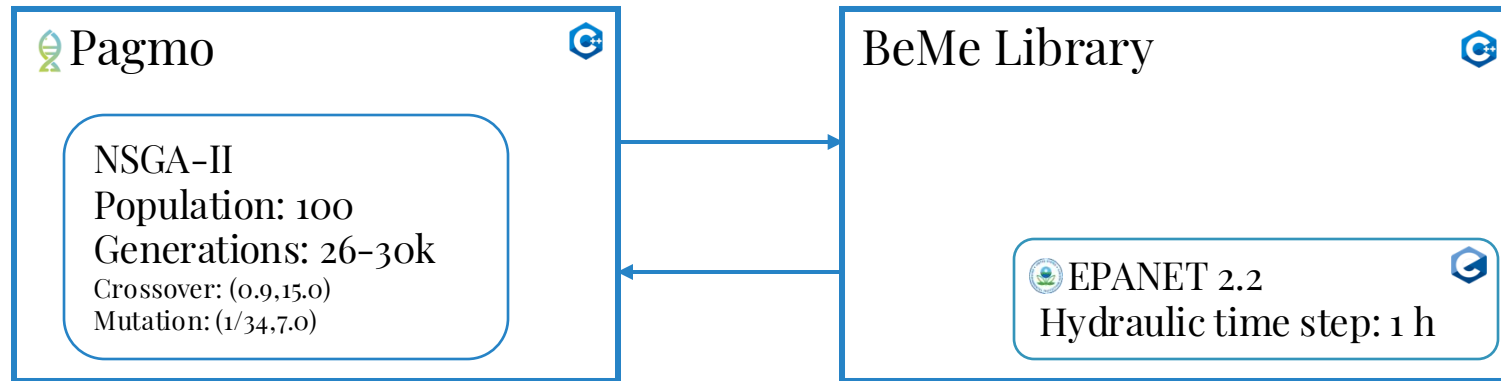




Two-phase looped



Experimental Settings



Design Decision Variables $[\langle x_1 | d_1 \rangle, \dots, \langle x_i | d_i \rangle, \dots, \langle x_{35} | d_{35} \rangle, \underbrace{d_{36}, \dots, d_j, \dots, d_{41}}_{\text{New pipes: diameter}}, \underbrace{\langle t_1^{loc} | t_1^{vol} \rangle, \langle t_2^{loc} | t_2^{vol} \rangle}_{\text{New tanks: (location'|volume)}}$

Existing pipes: (action|diameter)

Control Decision Variables $[\underbrace{z_1, \dots, z_i, \dots, z_{24}}_{\text{Pump station: \# active pumps at each hour}}]$

Implementation: [Zanutto, 2024](#)



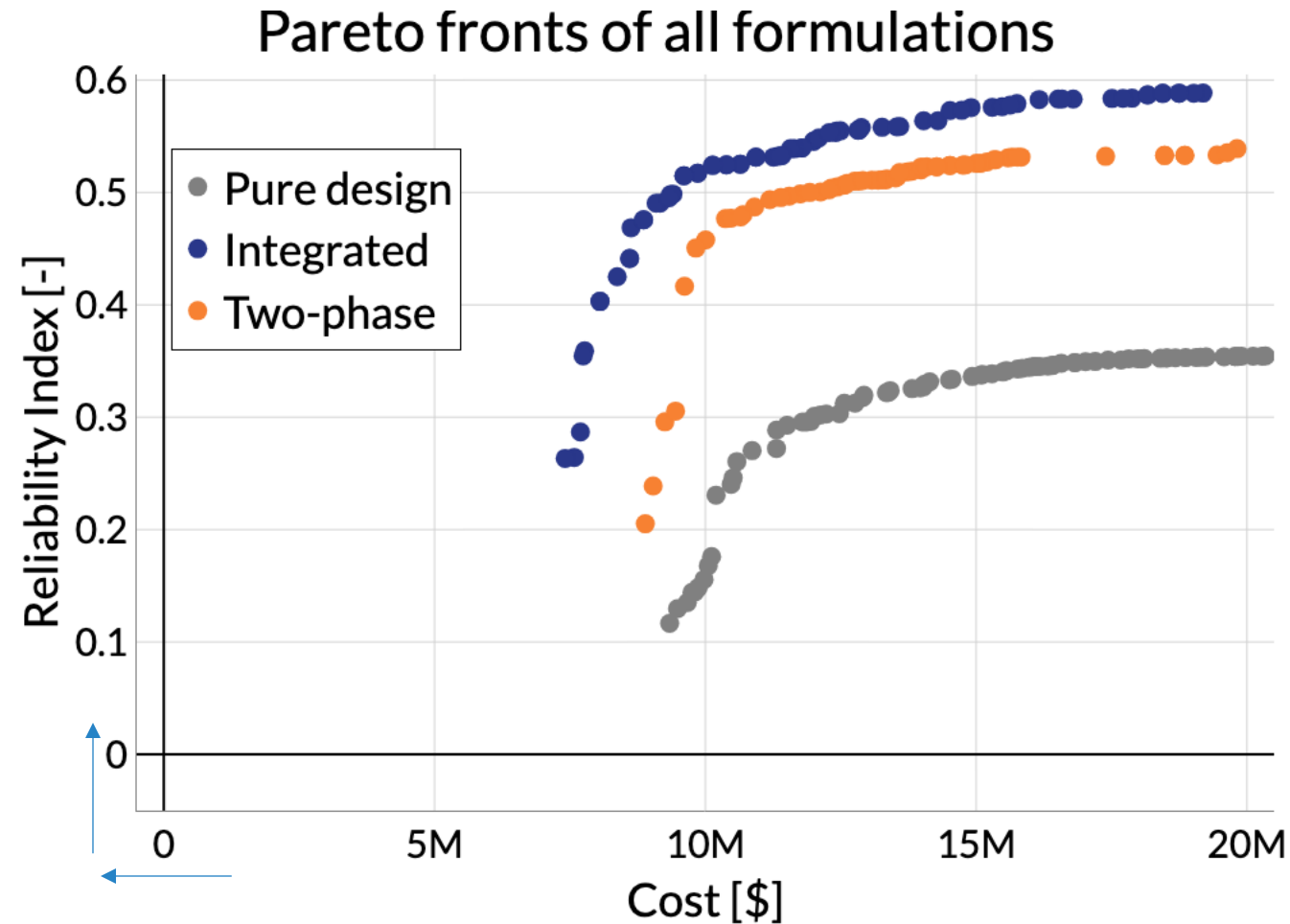
thanks to:

[Rossman, 2000](#)

[Deb et al., 2002](#)

[Biscani and Izzo, 2020](#)

Results



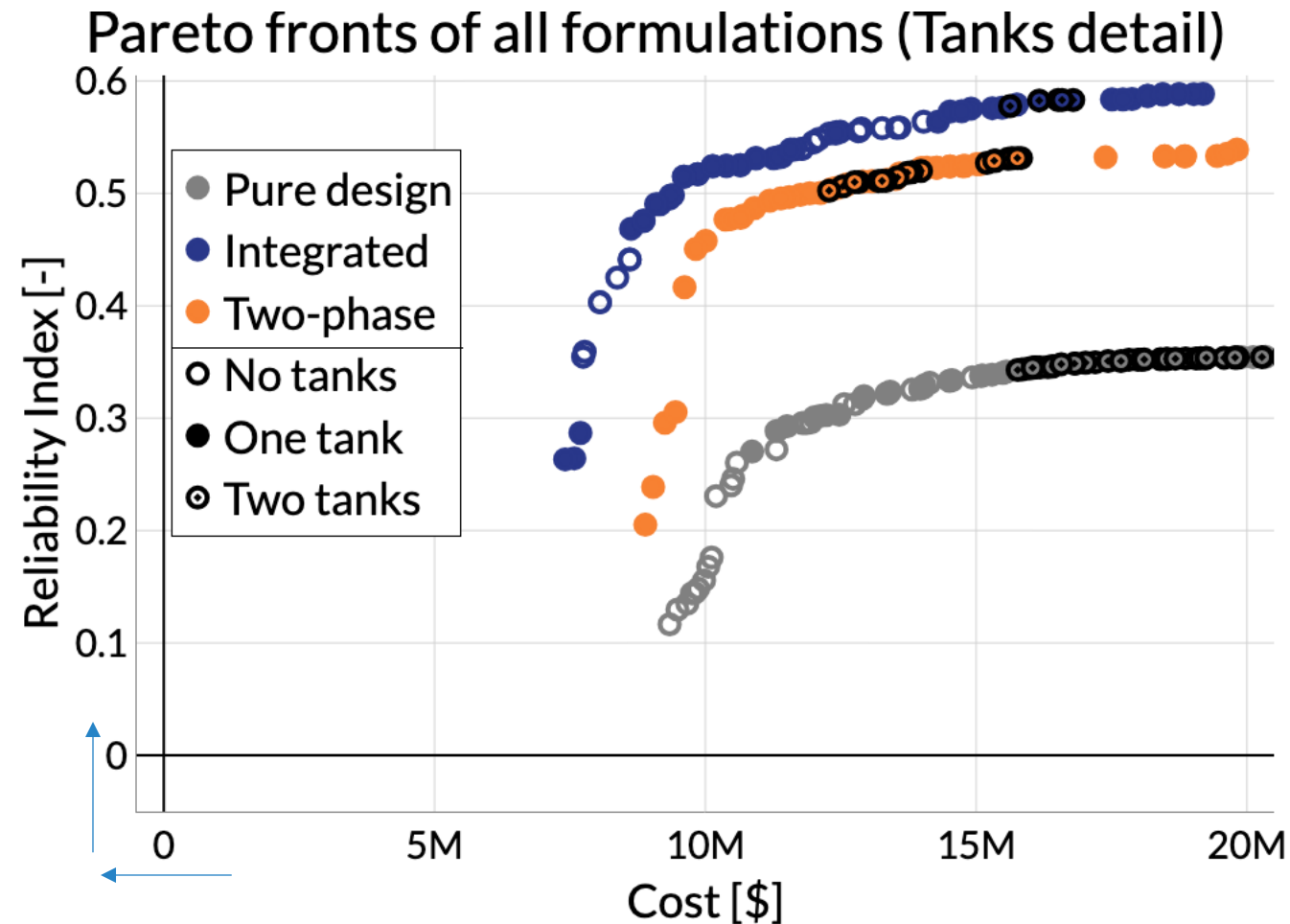
Integrated

- +180% at 10M
- +70% at 15M

Two-phase

- 150% at 10M
- 55% at 15M

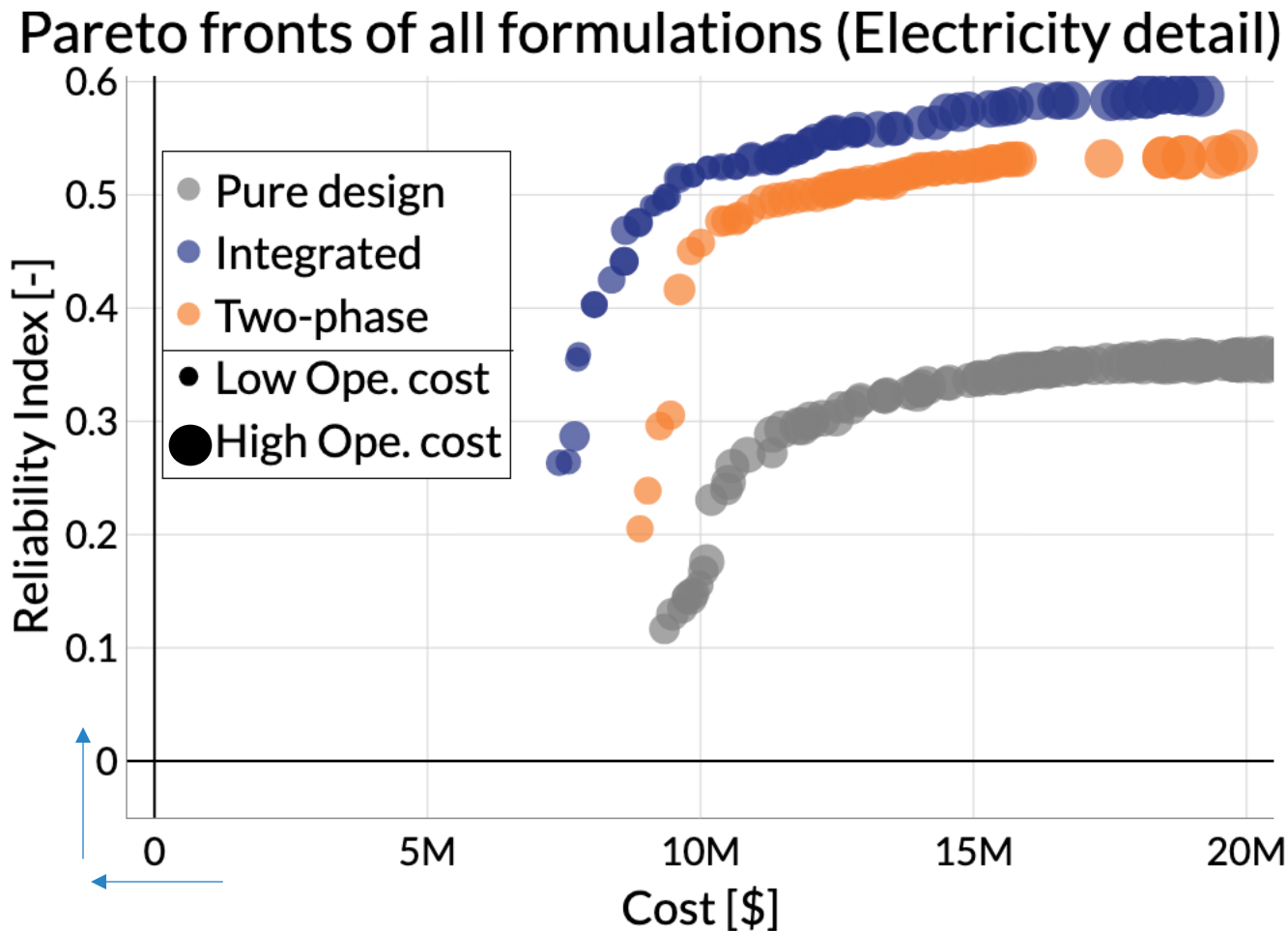
Results- detail-



There is no clear dependence on tanks for the joint approaches.

The pure design needs the tanks for high reliability!

Results- detail-

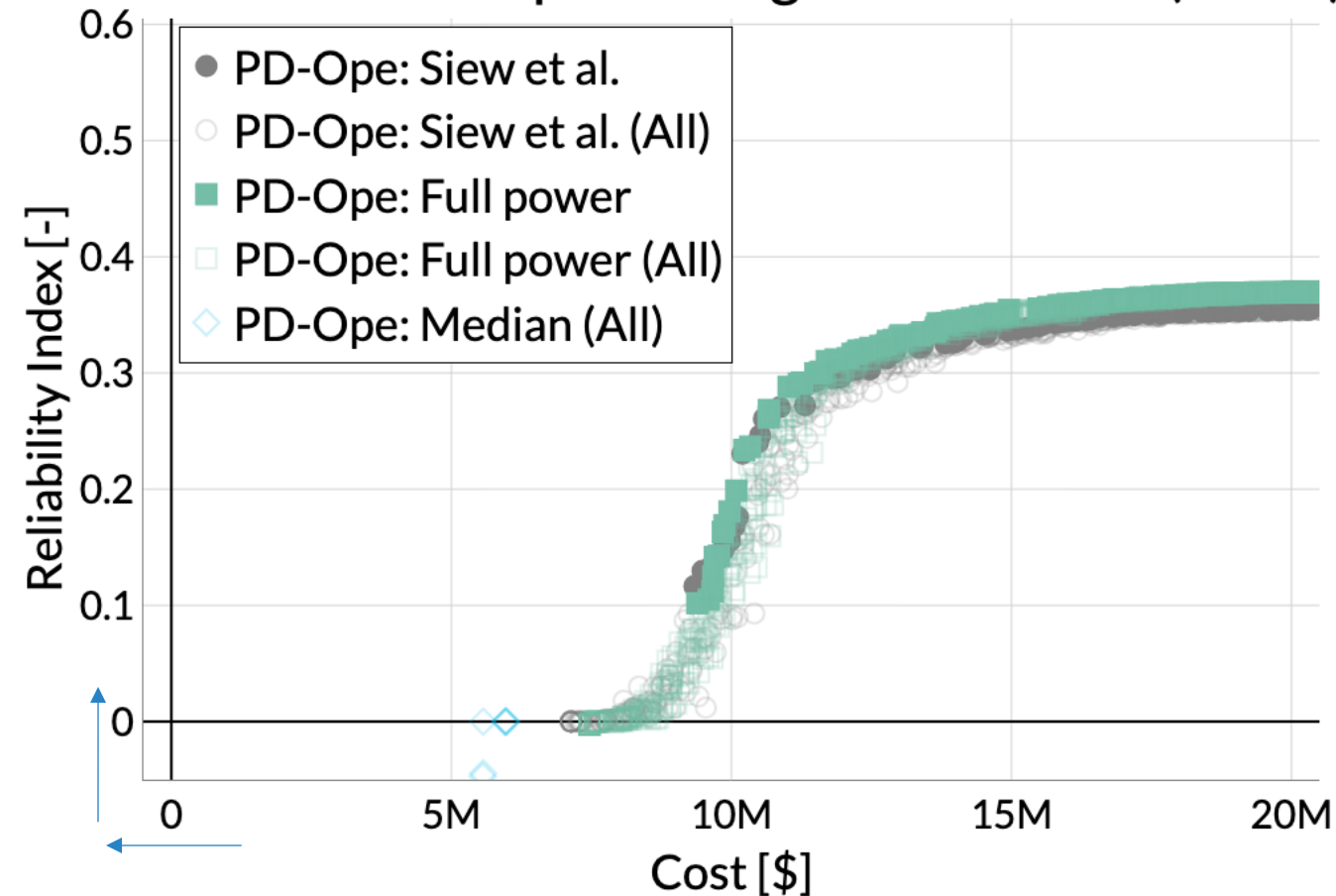


Similar operational costs.

Operations behaviour?

Results- detail-

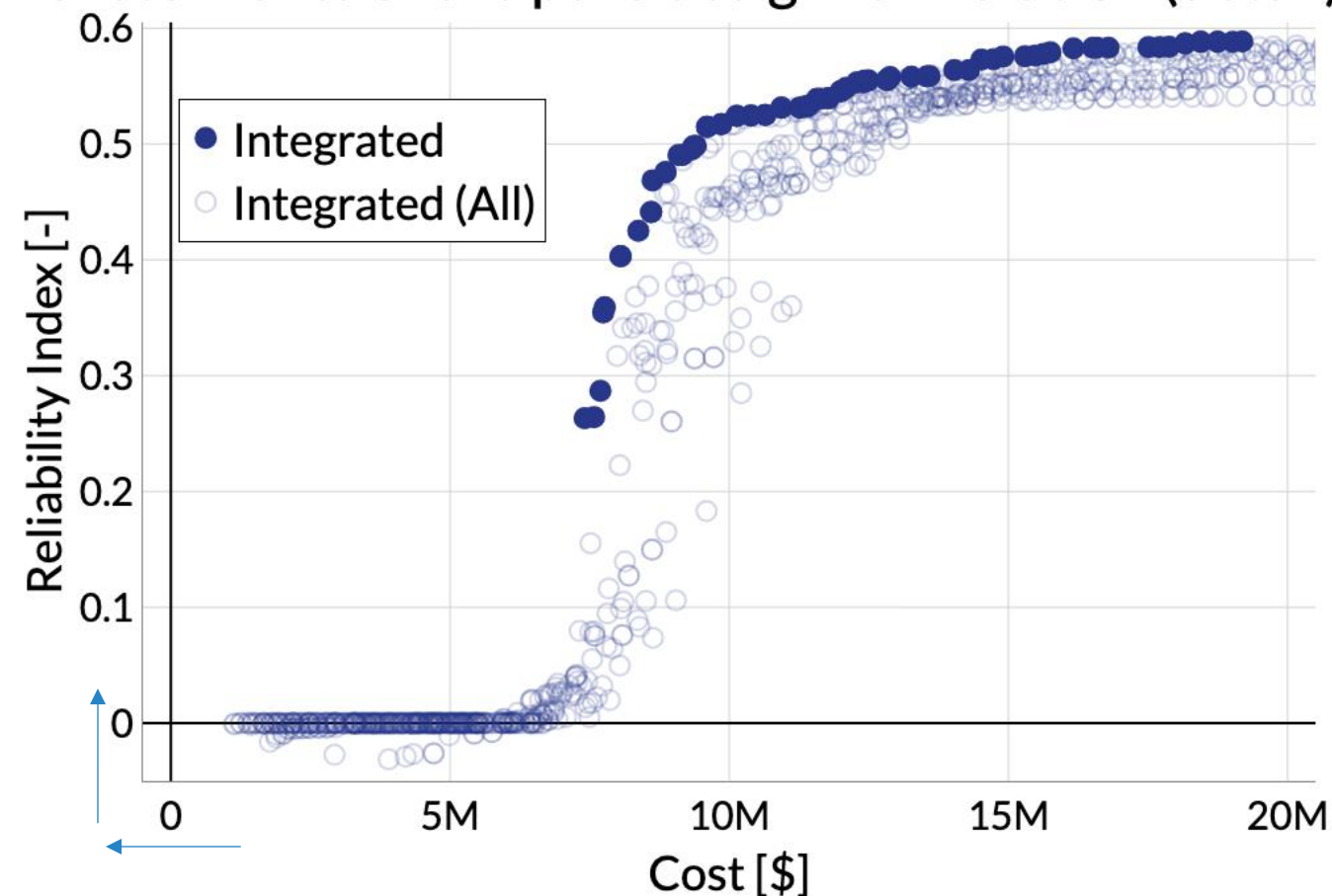
Pareto fronts of the pure design formulation (detail)



These are pretty standard results.

Results- detail-

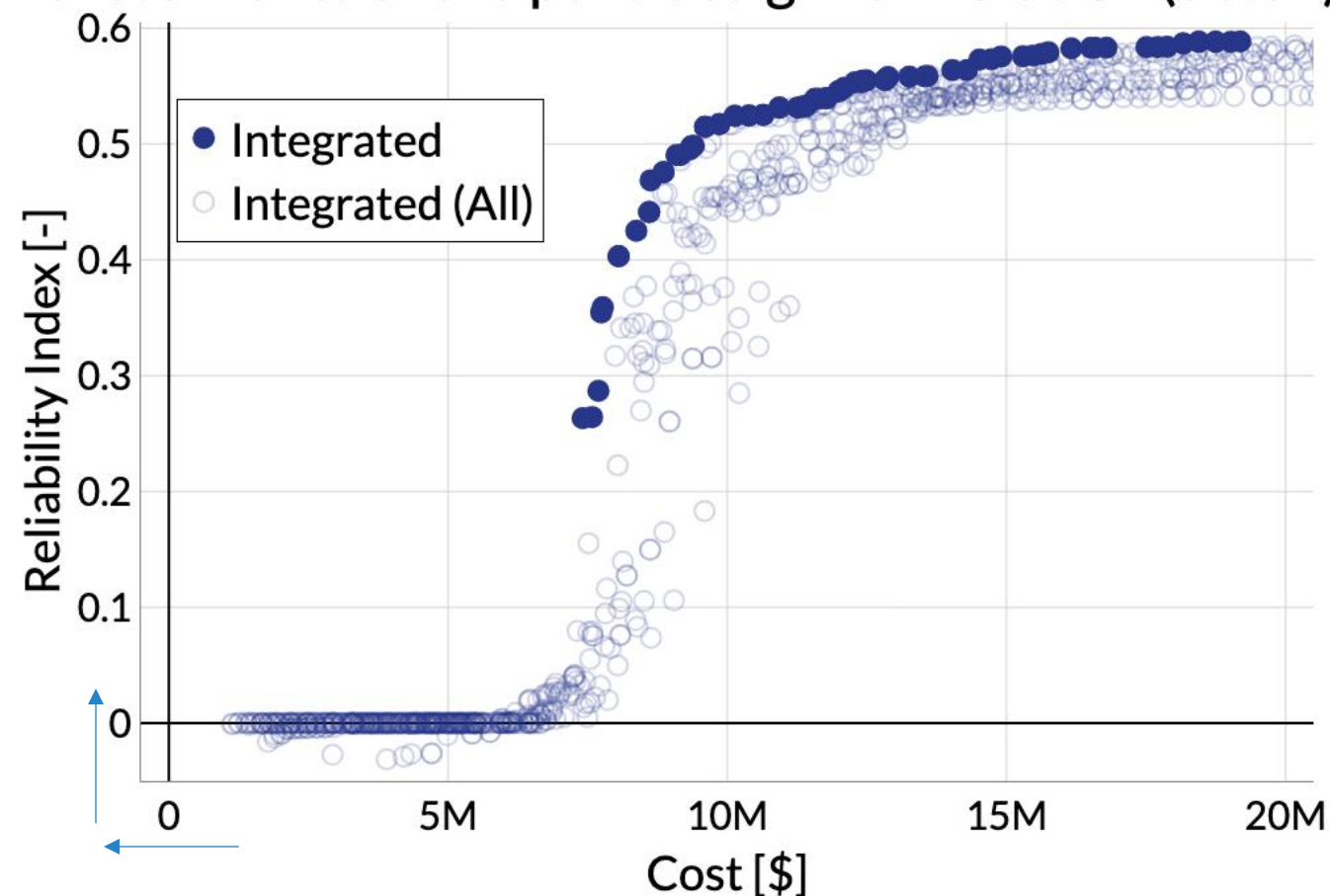
Pareto fronts of the pure design formulation (detail)



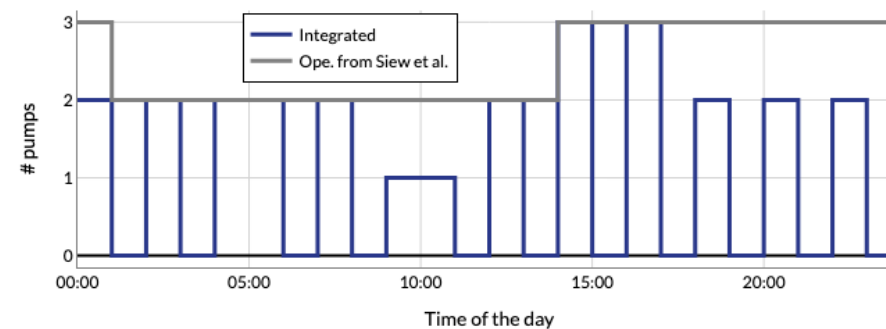
More local optimum.
Big attractor at $I_r = 0$.
Convergence issues.

Results- detail-

Pareto fronts of the pure design formulation (detail)

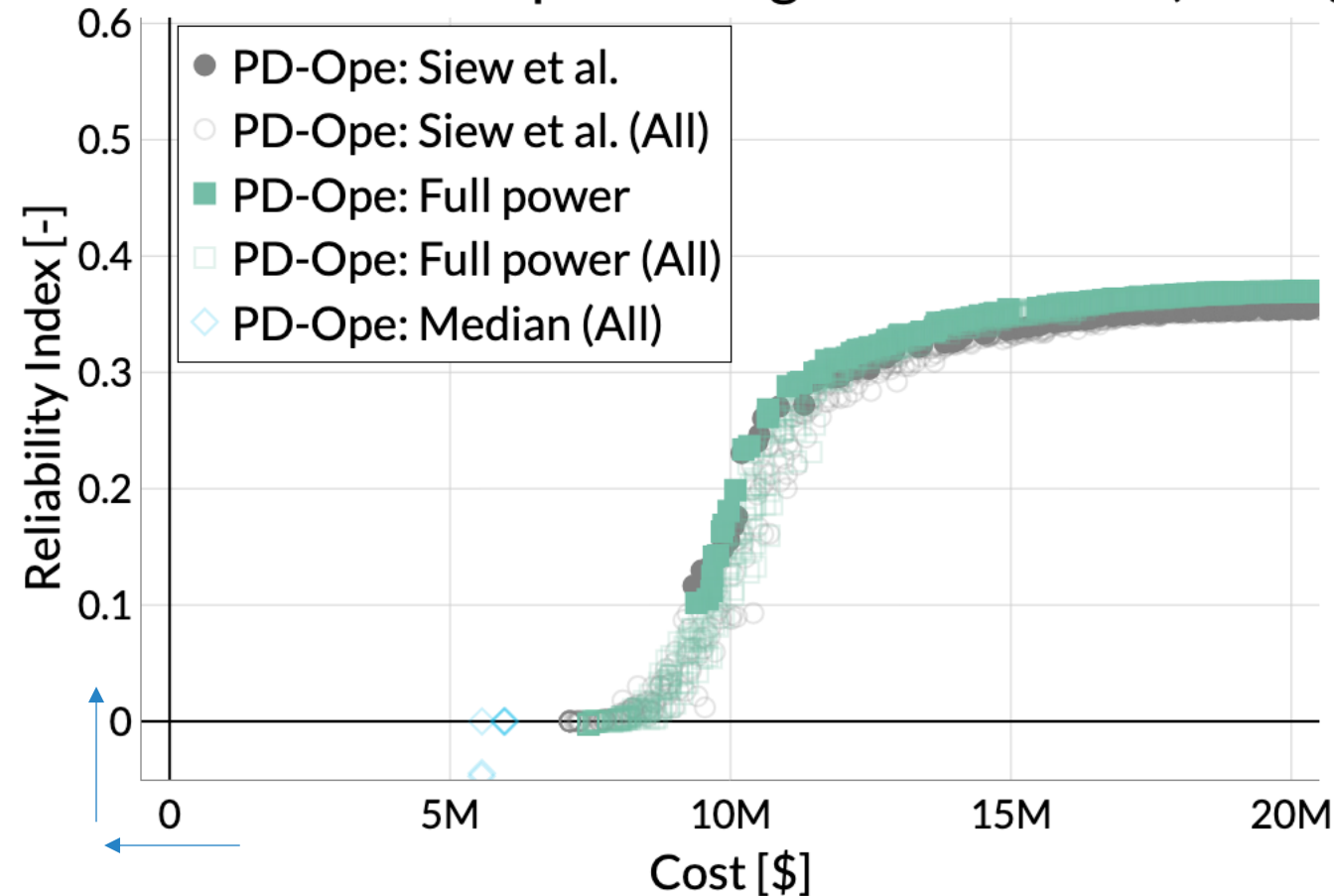


More local optimum.
Big attractor at $I_r = 0$.
Convergence issues.
How are operations optimised?



Results- detail-

Pareto fronts of the pure design formulation (detail)

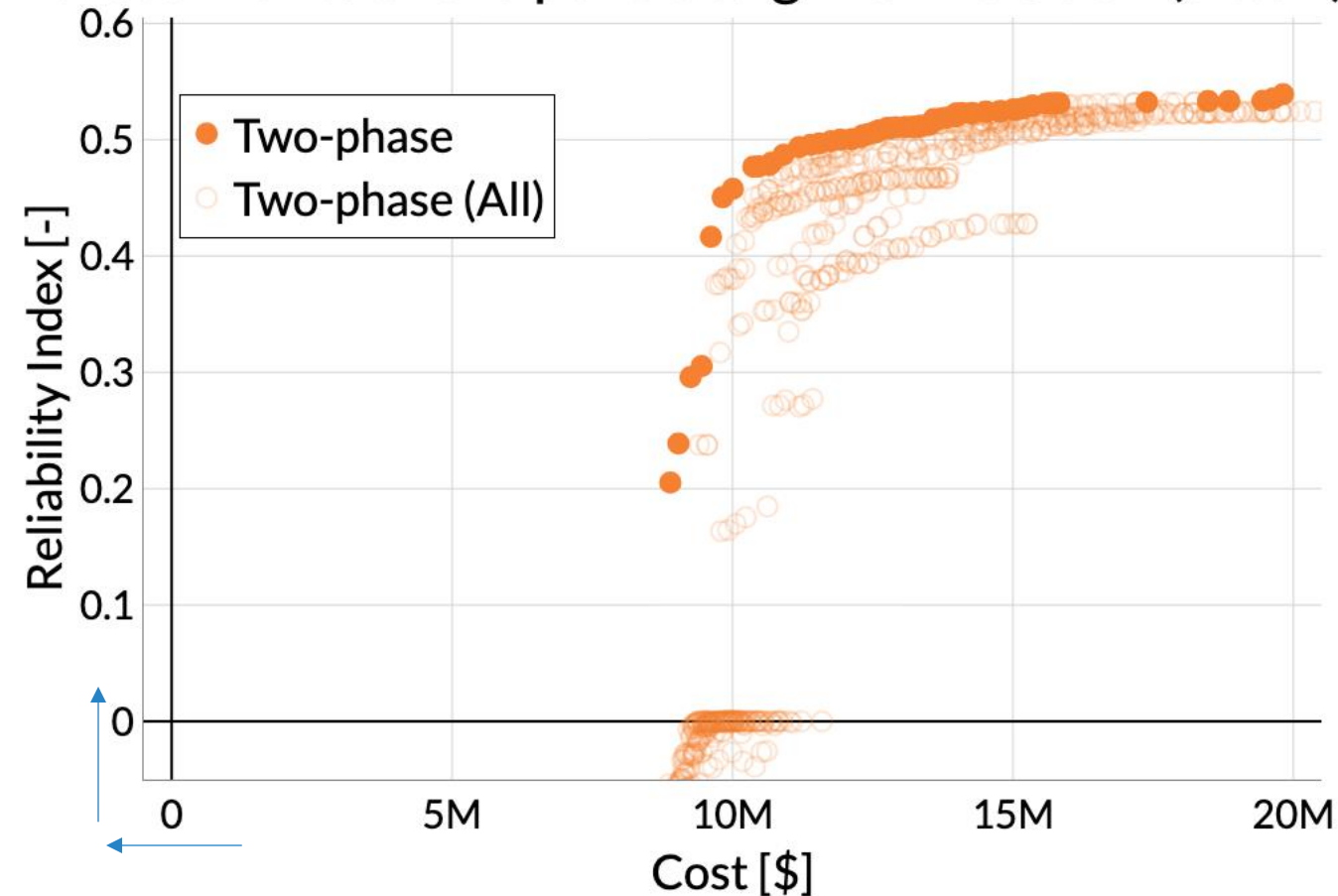


These are pretty standard results.

These optimised operations don't work!

Results- detail-

Pareto fronts of the pure design formulation (detail)



Higher stability.

Greater confusion.

Different attractor.

Material and Contacts

Source code (WIP)



Zenodo (results)



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References

Case study:

- Walski, T.M.; Brill, E.D.; Gessler, J.; Goulter, I.C.; Jeppson, R.M.; Lansey, K.; Lee, H.-L.; Liebman, J.C.; Mays, L.; Morgan, D.R.; et al. Battle of the Network Models: Epilogue. *J. Water Resour. Plan. Manag.* **1987**, *113*, 191–203, doi:10.1061/(ASCE)0733-9496(1987)113:2(191).
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Coding:

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Other:

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